# APPENDIX H WATER RESOURCES

# THIS PAGE INTENTIONALLY LEFT BLANK

# JURISDICTIONAL DETERMINATION COORDINATION

# THIS PAGE INTENTIONALLY LEFT BLANK



U.S Department of Transportation

Federal Aviation Administration

DEC - 5 2017

Ms. Sallie Diebolt, Chief Arizona Section Regulatory Branch U.S. Army Corps of Engineers 3636 North Central Avenue, Suite 900 Phoenix, Arizona 85012-1927

Dear Ms. Diebolt:

#### Tucson International Airport, Tucson, Arizona Clean Water Act Section 404 Preliminary Jurisdictional Delineation for the Proposed Airfield Safety Enhancement Project Reference Number: SPL201700643-KWG

The Federal Aviation Administration (FAA) is preparing a Draft Environmental Impact Statement (EIS) for the Proposed Airfield Safety Enhancement Project (ASEP) including real property transactions at Tucson International Airport (TUS) in Tucson, Pima County, Arizona (the Proposed Action). The FAA requests U.S. Army Corps of Engineers concurrence of the FAA's jurisdictional determination for purposes of the Clean Water Act Section 404 process.

The FAA is the lead federal agency for preparation of the EIS. The EIS is being prepared under the National Environmental Policy Act of 1969 (NEPA), its implementing regulations found at Title 40, Code of Federal Regulations (C.F.R.) Part 1500, as well as FAA's policies and procedures for complying with NEPA found in FAA Order 1050.1F, *Environmental Impacts: Policies and Procedures* and FAA Order 5050.4B, *NEPA Implementing Instructions for Airport Actions*. The United States Air Force (USAF) and the National Guard Bureau (NGB) are cooperating agencies as described under 40 C.F.R. § 1501.6 for preparation of the EIS.

The results of this coordination effort will be incorporated into the Draft EIS for the Proposed Action at TUS. As shown on the enclosed Exhibit 1, TUS is located on 8,343 acres in Tucson, Arizona in Pima County south of the City of Tucson central business district and near both Interstate 10 and Interstate 19. The USAF owned land, known as Air Force Plant 44 (AFP 44), is located along the southwest border of the Airport. Two study areas have been developed for the EIS as shown on Exhibit 2.

The General Study Area (GSA) is defined as the area where both direct and indirect impacts may result from the development of the Proposed Action and reasonable alternatives. The Detailed Study Area (DSA) is defined as the area where direct impacts may result from the Proposed Action and its alternatives. The DSA is comprised of

Western-Pacific Region Office of Airports 15000 Aviation Blvd, Suite 3012 Lawndale, CA 90261 several noncontiguous project sites within an area that is approximately four miles long and two miles wide. The Proposed Action is located in portions of Township 15S, Range 14E, Sections 17, 18, 19, 20, 21, 28, 29, 32, & 33 32.11252 -110.93930, WGS 84. Elevation within the DSA is 2,628 feet above mean sea level.

A pedestrian survey to identify Waters of the United States in the areas to be directly and indirectly impacted was conducted by Daniel Bunting and Chase Viorin of Harris Environmental, Inc. on June 29, 2017. The aerial photograph used for this preliminary jurisdictional delineation (PJD) was taken in October 2016. This photograph accurately depicts site conditions present during the pedestrian survey. The U.S. Fish and Wildlife Service National Wetland Inventory (NWI) maps were also reviewed and no mapped wetlands were found within the DSA. During the pedestrian survey, no potential wetlands were observed in the DSA, or immediately upstream or downstream.

In order to determine the area of a jurisdictional Waters of the U.S., the ordinary high water marks were used, including but not limited to, differences in vegetation, changes in soil characteristics, water stains, cut banks, and presence of litter and debris.

The following items are included for your review:

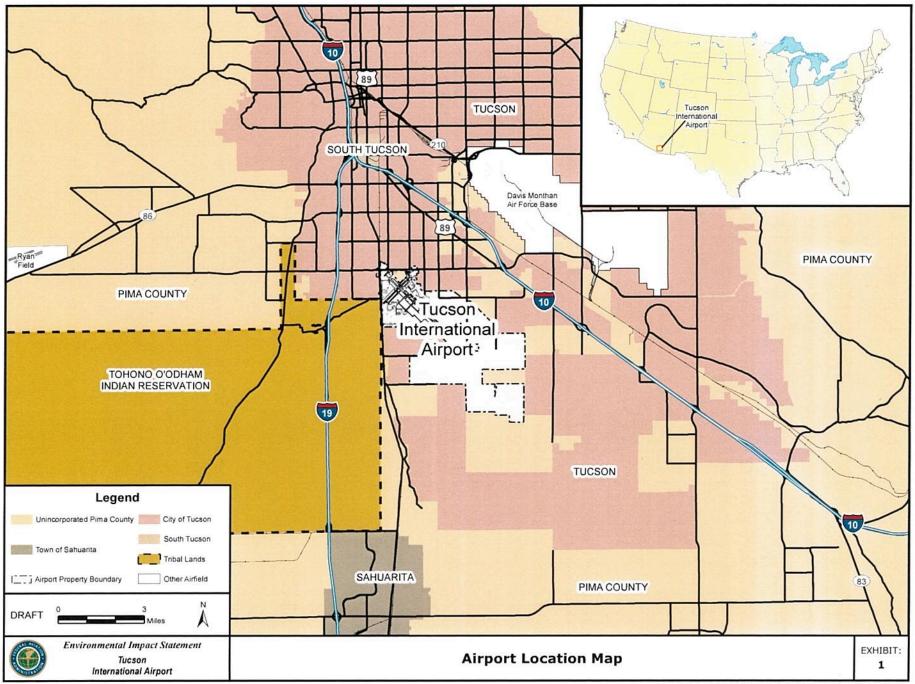
- Exhibit 1 Airport Location Map
- Exhibit 2 EIS Study Areas
- Preliminary Jurisdictional Determination Form
- Water Data Sheet
- Appendix A
  - o Ground Level Photographs
  - o (Aerial of areas surveyed with PJD overlaid for evaluation)

Please call me at 310/725-3615 or email me at <u>dave.kessler@faa.gov</u> if you have any questions or need additional information.

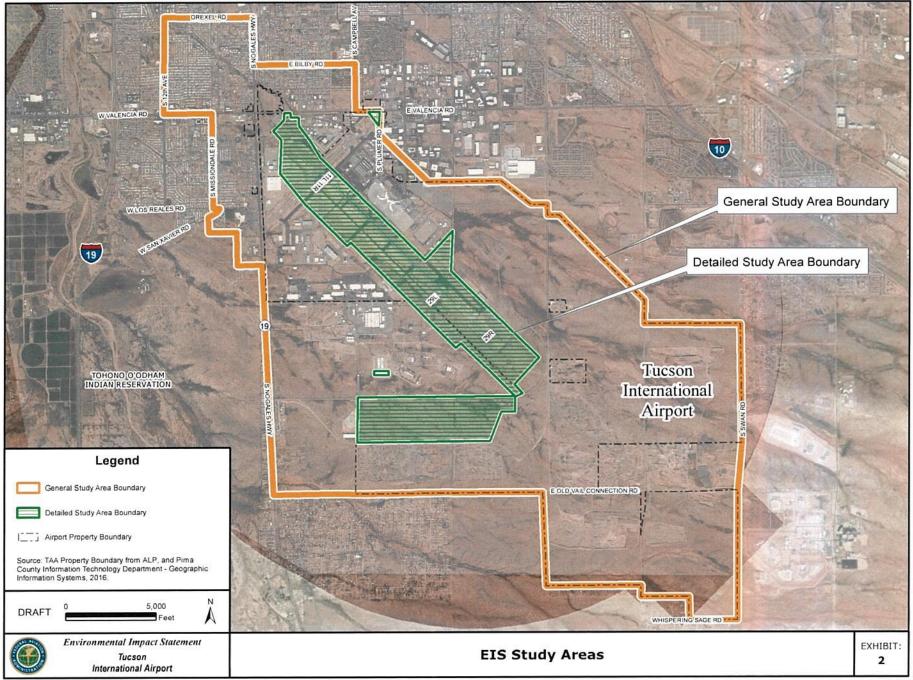
Sincerely,

David B. Kessler, M.A., AICP Regional Environmental Protection Specialist

Enclosures



10/9/2017 Y:\TUS\TUS-EIS\E-L&B Work Product\2-GIS\MXD\DOCUMENT\ 1-3\_Arport Location Map.mvd



#### PRELIMINARY JURISDICTIONAL DETERMINATION FORM

This preliminary JD finds that there "may be" waters of the United States on the subject project site, and identifies all aquatic features on the site that could be affected by the proposed activity, based on the following information:

District Office Los Angeles District File/ORM #		PJD Date: 6/29/17
State City/County Tucson/Pima	Name/	Daniel Bunting
Nearest Waterbody: Santa Cruz River	Address of	Harris Environmental Group, Inc. 650 N 6th Ave, Tucson, AZ 85716
Location: TRS, LatLong or UTM: Township 15S, Range 14E, Sections 17, 21, 28, 29, 32, & 33 32 11252 -110 93930 WGS 84	Person 18, 19, 20, PJD	650 N our Ave, Tueson, AZ 85716
Identify (Estimate) Amount of Waters in the Review Area:	Name of Any Water Bodies	
Non-Wetland Waters:         Stream Flow:           linear ft         width         17.57         acres         Ephemeral	on the Site Identified as	Tidal: NA on-Tidal: NA
Wetlands:         0         acre(s)         Cowardin Class:         N/A	<ul> <li>☐ Office (Desk) Determina</li> <li>☐ Field Determination:</li> </ul>	ation Date of Field Trip:
SUPPORTING DATA: Data reviewed for preliminary JD and requested, appropriately reference sources below):	) (check all that apply - checked	items should be included in case file and, where checked
<ul> <li>☞ Maps, plans, plots or plat submitted by or on behalf of the □ Office concurs with data sheets/delineation □ Office does not concur with data sheets/deli</li> <li>□ Data sheets prepared by the Corps</li> <li>□ Corps navigable waters' study: □</li> <li>□ U.S. Geological Survey Hydrologic Atlas: □ USGS NHD data. □ USGS 8 and 12 digit HUC maps.</li> <li>☞ U.S. Geological Survey map(s). Cite quad name: □</li> <li>□ USDA Natural Resources Conservation Service Soil □ National wetlands inventory map(s). Cite name: □</li> <li>□ State/Local wetland inventory map(s). Cite name: □</li> <li>□ FEMA/FIRM maps: □</li> <li>□ 100-year Floodplain Elevation is: □</li> <li>□ Photographs: ☞ Aerial (Name &amp; Date): 10/10/2016 M</li> <li>□ Other (Name &amp; Date): □</li> <li>□ Other information (please specify): □</li> </ul>	e applicant/consultant. report. ineation report. ueson & Tueson SW Survey. Citation:	
Signature and Date of Regulatory Project Manager (REQUIRED)		Person Requesting Preliminary JD
EXPLANATION OF PRELIMINARY AND APPROVED JURISDICTIONAL DD 1. The Corps of Engineers believes that there may be jurisdictional waters of the Unit hereby advised of his or her option to request and obtain an approved jurisdictional de has declined to exercise the option to obtain an approved JD in this instance and at this 2. In any circumstance where a permit applicant obtains an individual permit, or a Nat or requests verification for a non-reporting NWP or other general permit, and the per following: (1) the permit applicant has elected to seek a permit authorization based on the option to request an approved JD before accepting the terms and conditions of compensatory mitigation being required or different special conditions; (3) that the ap other general permit authorization; (4) that the applicant can accept a permit authoriz requirements the Corps has determined to be necessary; (5) that undertaking any activ acceptance of the use of the preliminary JD, but that either form of JD will be proce undertaking any activity in reliance on any form of Corps permit authorization based on that activity are jurisdictional waters of the United States, and precludes any challeng appeal or in any Edecal court: and (7) whether the amilicant clect to use either an	ETERMINATIONS: ted States on the subject site, and the p termination (JD) for that site. Neverthe time. tionwide General Permit (NWP) or othe mit applicant has not requested an app a preliminary JD, which does not mak the permit authorization, and that basis pplicant has the right to request an indi- zation and thereby agree to comply wit vity in reliance upon the subject permit ressed as soon as is practicable; (6) acce on a preliminary JD constitutes agreeme ge to such jurisdiction in any administr	ermit applicant or other affected party who requested this preliminary JD is eless, the permit applicant or other person who requested this preliminary JD er general permit verification requiring "preconstruction notification" (PCN), roved JD for the activity, the permit applicant is hereby made aware of the e an official determination of jurisdictional waters; (2) that the applicant has ing a permit authorization on an approved JD could possibly result in less vidual permit rather than accepting the terms and conditions of the NWP or h all the terms and conditions of that permit, including whatever mitigation authorization without requesting an approved JD constitutes the applicant's epting a permit authorization (e.g., signing a proffered individual permit) or in that all wetlands and other water bodies on the site affected in any way by ative or judicial compliance or enforcement action, or in any administrative

that activity are jurisdictional waters of the United States, and precludes any challenge to such jurisdiction in any administrative or judicial compliance or enforcement action, or in any administrative appeal or in any Federal court; and (7) whether the applicant elects to use either an approved JD or a preliminary JD, that JD will be processed as soon as is practicable. Further, an approved JD, a proffered individual permit (and all terms and conditions contained therein), or individual permit denial can be administratively appealed pursuant to 33 C.F.R. Part 331, and that in any administrative appeal, jurisdictional issues can be raised (see 33 C.F.R. 331.5(a)(2)). If, during that administrative appeal, it becomes necessary to make an official determination whether CWA jurisdiction exists over a site, or to provide an official delineation of jurisdictional waters on the site, the Corps will provide an approved JD to accomplish that result, as soon as is practicable.

# PRELIMINARY JURISDICTIONAL DETERMINATION FORM

				Appendix A - Si	tes		
istrict (	Office		File/ORM #	ŧ		 PJD Date:	June 29th, 2017
ate	Cit	y/County Tucson	n/Pima	Pe	rson Requestinq PJ	D Daniel Buntir	ıg
	Site Number	Latitude	Longitude	Cowardin Class	Est. Amount of Aquatic Resourc in Review Area	e Class o Aquatic Ro	5.5r
	1	32.10477	-110.92903	Riverine	6717	Non-Section 1	0 non-wetlai
	2	32.09004	-110.93028	Riverine	53359	Non-Section 1	0 non-wetlaı
	3	32.09011	-110.92140	Riverine	10996	Non-Section 1	0 non-wetlaı
			-	n/a	-	Non-Section 1	0 non-wetlaı
			-	n/a		Non-Section 1	0 non-wetlaı
			-	n/a	· · · · · · · · · · · · · · · · · · ·	Non-Section 1	<u> </u>

Notes:

Potential impacts to WOUS being analyzed in the EIS. Findings and any potential mitigation measures will be coordinated with USACE.

#### Water Data Sheet

Waters_Name	Cowardin_Code	HGM_Code	Measurement_Type	Amount
Hughes Wash ephemeral tributary 1	R4SB4	RIVERINE	Area	1.7
Hughes Wash ephemeral tributary 2	R4SB4	RIVERINE	Area	13.2
Hughes Wash	R4SB4	RIVERINE	Area	2.7

Units	Waters_Type	Latitude	Longitude	Local_Waterway
ACRE	DELINEATE	32.10477	-110.92903	Hughes Wash ephemeral tributary 1
ACRE	DELINEATE	32.09004	-110.93028	Hughes Wash ephemeral tributary 2
ACRE	DELINEATE	32.09011	-110.9214	Hughes Wash

## **APPENDIX A – Project Ground Level Photographs**



Photo 1: Shallow drainage, Hughes Trib 1

Photo 3: 6 x 3-ft culvert pipes (inflow)



Photo 5: Culvert (inflow)



Photo 2: Exit of drainage toward culvert

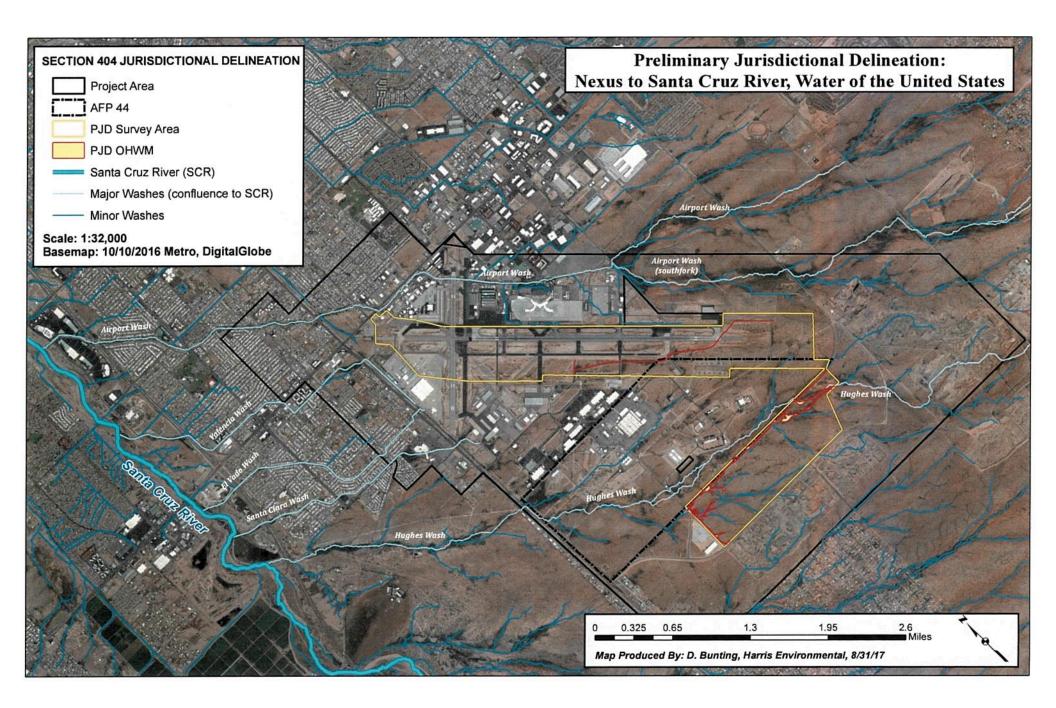


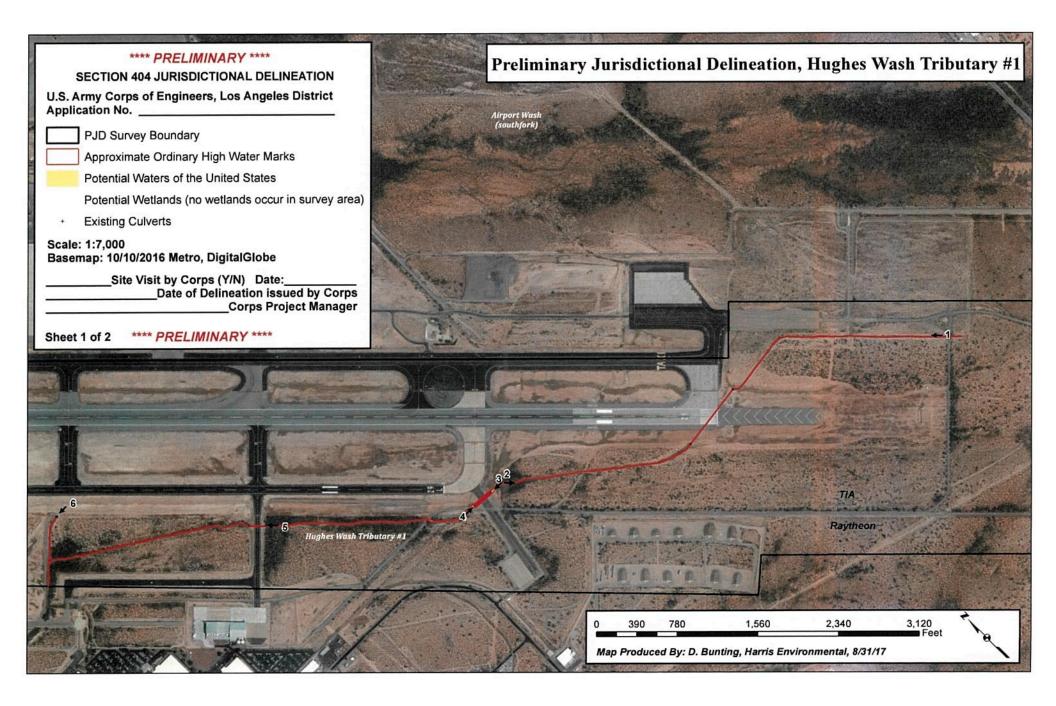
Photo 4: 6 x 3-ft culvert pipes (outflow)

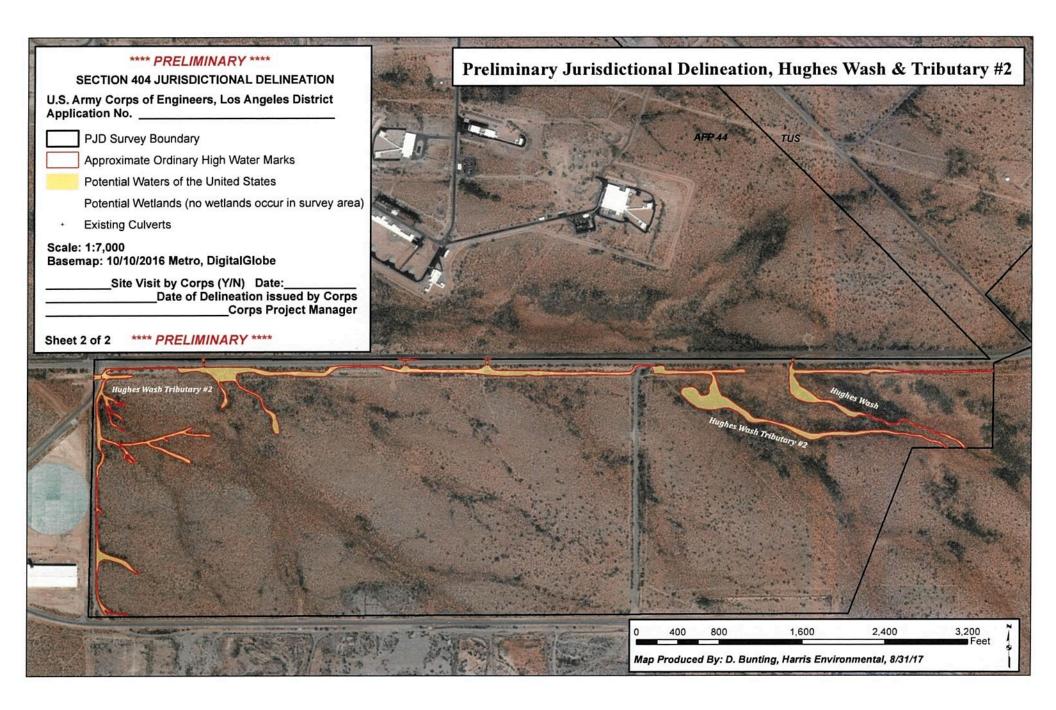


Photo 6: Culvert (outflow)









THIS PAGE INTENTIONALLY LEFT BLANK



#### DEPARTMENT OF THE ARMY LOS ANGELES DISTRICT, U.S. ARMY CORPS OF ENGINEERS 3636 N CENTRAL AVE SUITE 900 PHOENIX AZ 85012-1939

December 14, 2017

SUBJECT: Preliminary Jurisdictional Determination

David Kessler Federal Aviation Administration Western-Pacific Region, Office of Airports 15000Aviation Blvd. Suite 3012 Lawndale, California 90261

Dear Mr. Kessler:

I am responding to your request (File No. SPL-2017-00643-KWG) dated December 5, 2017 for a preliminary Department of the Army jurisdictional determination (JD) for the proposed Tucson International Airport's Airfield Safety Enhancement Project site midpoint Lat/Long: 32.10477/-110.92903 located within the city of Tucson, Pima County, Arizona.

The Corps' evaluation process for determining whether a Department of the Army permit is needed involves two tests. If both tests are met, a permit would likely be required. The first test determines whether the proposed project is located within the Corps' geographic jurisdiction (i.e., it is within a water of the United States). The second test determines whether as proposed, the project involves a regulated activity under Corps' authority, i.e., Section 10 of the Rivers and Harbors Act of 1899, Section 404 of the Clean Water Act, or Section 103 of the Marine Protection Research and Sanctuaries Act. The determination in this letter pertains only to the question of geographic jurisdiction.

Based on available information, I have preliminarily determined waters of the U.S. may be present on the proposed Tucson International Airport's Airfield Safety Enhancement Project site in the approximate locations noted on the enclosed map. The basis for this finding may be found on the enclosed Preliminary Jurisdictional Determination (JD) form. Preliminary JDs are non-binding indications of the presence of waters of the U.S., including wetlands, on a parcel. Preliminary JDs are advisory in nature and may not be appealed.

This determination was conducted to identify the extent of the Corps' Clean Water Act jurisdiction on the proposed Tucson International Airport's Airfield Safety Enhancement Project site identified in your request. This determination may not be valid for the wetland conservation provisions of the Food Security Act of 1985. If you or your tenant are USDA program participants, or anticipate participation in USDA programs, you should request a certified wetland determination from the local office of the Natural Resources Conservation Service, prior to starting work. Thank you for participating in the regulatory program. If you have any questions, please contact Kevin Grove at (602) 230-6957 or via e-mail at Kevin.W.Grove@usace.army.mil. Please help me to evaluate and improve the regulatory experience for others by completing the <u>customer survey</u> form at <u>http://corpsmapu.usace.army.mil/cm\_apex/f?p=regulatory\_survey</u>.

Sincerely,

Michael W. Langley Senior Project Manager

SPL201700643 Enclosure(s)

- 1. 20171211-201700643-FAA-PJD-Maps
- 2. 20171211\_201700643\_FAA\_PJDform

#### NOTIFICATION OF ADMINISTRATIVE APPEAL OPTIONS AND PROCESS AND REQUEST FOR APPEAL

Applicant: Federal Aviation Administration, Attn: Mr. David File No.: SPL-2017-00643-KWG	Date: December 14,
Kessler File No.: SFL-2017-00043-KWG	2017
Attached is:	See Section below
INITIAL PROFFERED PERMIT (Standard Permit or Letter of permission)	А
PROFFERED PERMIT (Standard Permit or Letter of permission)	В
PERMIT DENIAL	С
APPROVED JURISDICTIONAL DETERMINATION	D
X PRELIMINARY JURISDICTIONAL DETERMINATION	Е

SECTION I - The following identifies your rights and options regarding an administrative appeal of the above decision. Additional information may be found at *http://www.usace.army.mil/cecw/pages/reg\_materials.aspx* or Corps regulations at 33 CFR Part 331.

A: INITIAL PROFFERED PERMIT: You may accept or object to the permit.

• ACCEPT: If you received a Standard Permit, you may sign the permit document and return it to the district engineer for final authorization. If you received a Letter of Permission (LOP), you may accept the LOP and your work is authorized. Your signature on the Standard Permit or acceptance of the LOP means that you accept the permit in its entirety, and waive all rights to appeal the permit, including its terms and conditions, and approved jurisdictional determinations associated with the permit.

• OBJECT: If you object to the permit (Standard or LOP) because of certain terms and conditions therein, you may request that the permit be modified accordingly. You must complete Section II of this form and return the form to the district engineer. Your objections must be received by the district engineer within 60 days of the date of this notice, or you will forfeit your right to appeal the permit in the future. Upon receipt of your letter, the district engineer will evaluate your objections and may: (a) modify the permit to address all of your concerns, (b) modify the permit to address some of your objections, or (c) not modify the permit having determined that the permit should be issued as previously written. After evaluating your objections, the district engineer will send you a proffered permit for your reconsideration, as indicated in Section B below.

B: PROFFERED PERMIT: You may accept or appeal the permit

• ACCEPT: If you received a Standard Permit, you may sign the permit document and return it to the district engineer for final authorization. If you received a Letter of Permission (LOP), you may accept the LOP and your work is authorized. Your signature on the Standard Permit or acceptance of the LOP means that you accept the permit in its entirety, and waive all rights to appeal the permit, including its terms and conditions, and approved jurisdictional determinations associated with the permit.

• APPEAL: If you choose to decline the proffered permit (Standard or LOP) because of certain terms and conditions therein, you may appeal the declined permit under the Corps of Engineers Administrative Appeal Process by completing Section II of this form and sending the form to the division engineer (address on reverse). This form must be received by the division engineer within 60 days of the date of this notice.

C: PERMIT DENIAL: You may appeal the denial of a permit under the Corps of Engineers Administrative Appeal Process by completing Section II of this form and sending the form to the division engineer (address on reverse). This form must be received by the division engineer within 60 days of the date of this notice.

D: APPROVED JURISDICTIONAL DETERMINATION: You may accept or appeal the approved JD or provide new information.

• ACCEPT: You do not need to notify the Corps to accept an approved JD. Failure to notify the Corps within 60 days of the date of this notice, means that you accept the approved JD in its entirety, and waive all rights to appeal the approved JD.

• APPEAL: If you disagree with the approved JD, you may appeal the approved JD under the Corps of Engineers Administrative Appeal Process by completing Section II of this form and sending the form to the division engineer (address on reverse). This form must be received by the division engineer within 60 days of the date of this notice.

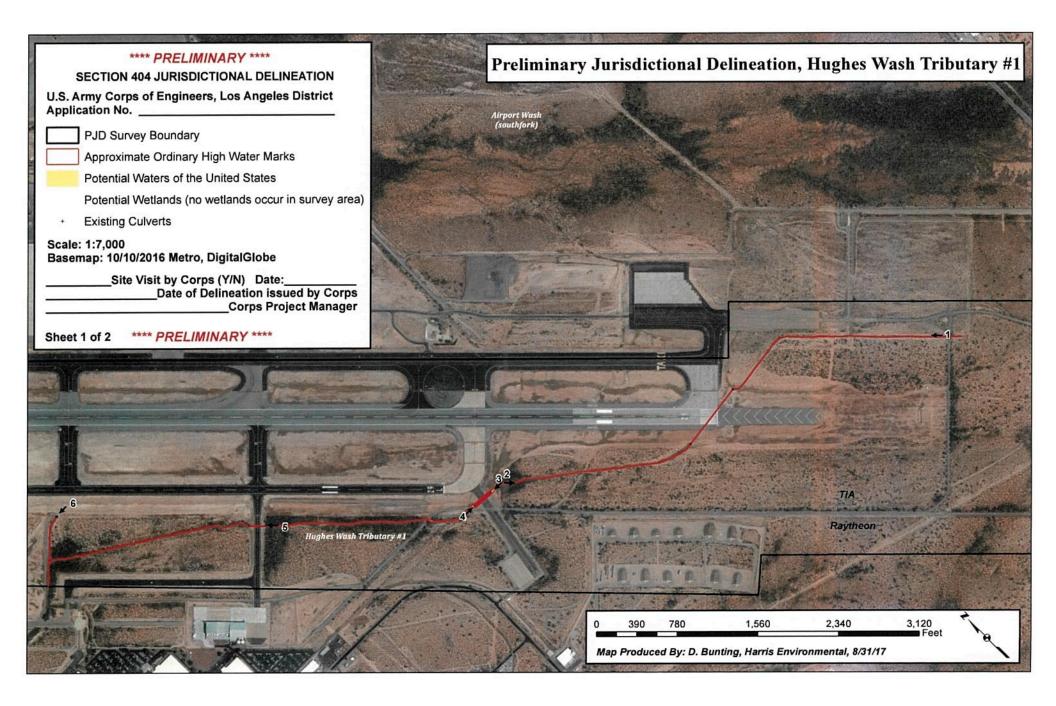
E: PRELIMINARY JURISDICTIONAL DETERMINATION: You do not need to respond to the Corps regarding the preliminary JD. The Preliminary JD is not appealable. If you wish, you may request an approved JD (which may be appealed), by contacting the Corps district for further instruction. Also you may provide new information for further consideration by the Corps to reevaluate the JD.

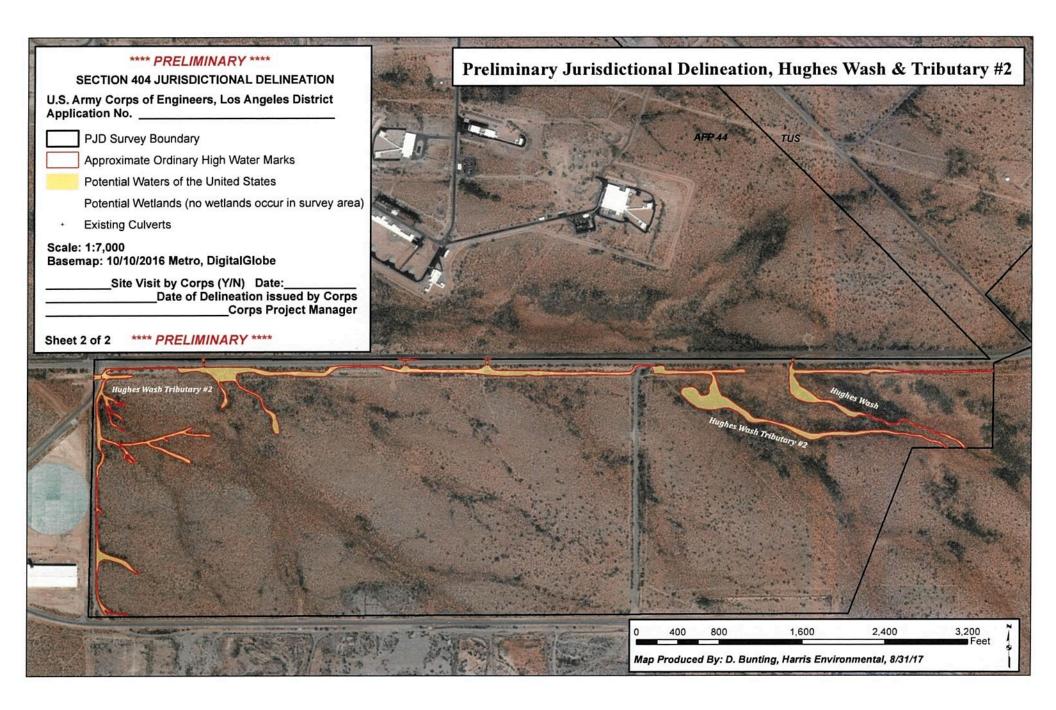
#### SECTION II - REQUEST FOR APPEAL or OBJECTIONS TO AN INITIAL PROFFERED PERMIT

**REASONS FOR APPEAL OR OBJECTIONS**: (Describe your reasons for appealing the decision or your objections to an initial proffered permit in clear concise statements. You may attach additional information to this form to clarify where your reasons or objections are addressed in the administrative record.)

ADDITIONAL INFORMATION: The appeal is limited to a review of the administrative record, the Corps memorandum for the record of the appeal conference or meeting, and any supplemental information that the review officer has determined is needed to clarify the administrative record. Neither the appellant nor the Corps may add new information or analyses to the record. However, you may provide additional information to clarify the location of information that is already in the administrative record.

POINT OF CONTACT FOR QUESTIONS OR INFORMATION:				
If you have questions regarding this decision and/or the appeal process you	If you only have questions regarding	the appeal process you		
may contact:	may also contact:			
	Thomas J. Cavanaugh			
U.S. Army Corps of Engineers	Administrative Appeal Review Officer			
	U.S. Army Corps of Engineers			
Phone: (602) 230-6957, FAX 916-557-7803	South Pacific Division			
Email: Kevin.W.Grove@usace.army.mil	1455 Market Street, 2052B			
	San Francisco, California 94103-1399			
	Phone: 415-503-6574, FAX 415-503-6646)			
Email: Thomas.J.Cavanaugh@usace.army.n		@usace.army.mil		
RIGHT OF ENTRY: Your signature below grants the right of entry to	Corps of Engineers personnel, and	any government		
consultants, to conduct investigations of the project site during the cour	se of the appeal process. You will	be provided a 15 day		
notice of any site investigation, and will have the opportunity to particip	pate in all site investigations.			
	Date:	Telephone		
		number:		
Signature of appellant or agent.				





## PRELIMINARY JURISDICTIONAL DETERMINATION FORM

This preliminary JD finds that there "may be" waters of the United States on the subject project site, and identifies all aquatic features on the site that could be affected by the proposed activity, based on the following information:

District Office Los Angeles District File/ORM #			PJD Date: 6/29/17	
State City/County Tueson/Pima		Name/	Daniel Bunting	
Nearest Waterbody: Santa Cruz River		Address of Person	Harris Environmental Group, Inc. 650 N 6th Ave, Tucson, AZ 85716	
Location: TRS, LatLong or UTM: Township 15S, Range 14E, Sections 17, 21, 28, 29, 32, & 33 32 11252 -110 93930 WGS 84	18, 19, 20,	Requesting PJD		
Identify (Estimate) Amount of Waters in the Review Area: <u>Non-Wetland Waters:</u> Stream Flow Innear ft width 17.57 acres Ephemeral	Name of Any Non the Site Ic Section 10	lentified as	Tidal NA n-Tidal NA	
Wetlands: 0 acre(s) Cowardin N/A		(Desk) Determination	ation Date of Field Trip	
SUPPORTING DATA: Data reviewed for preliminary JD and requested, appropriately reference sources below)	) (check all that a	ipply - checked i	tems should be included in case file and, where checked	
<ul> <li>✓ Maps, plans, plots or plat submitted by or on behalf of th </li> <li>✓ Data sheets prepared/submitted by or on behalf of th </li> <li>✓ Office concurs with data sheets/delineation </li> <li>✓ Office does not concur with data sheets/del </li> <li>✓ Data sheets prepared by the Corps </li> <li>✓ Corps navigable waters' study: </li> <li>✓ U.S. Geological Survey Hydrologic Atlas: </li> <li>✓ USGS NHD data. </li> <li>✓ USGS 8 and 12 digit HUC maps. </li> <li>✓ USDA Natural Resources Conservation Service Soil </li> <li>✓ National wetlands inventory map(s). Cite name: </li> <li>✓ State/Local wetland inventory map(s): </li> <li>✓ FEMA/FIRM maps: </li> <li>✓ Other (Name &amp; Date): 100/10/2016 N </li> <li>✓ Other (Name &amp; Date): Ground leve </li> <li>✓ Previous determination(s). File no. and date of respondence of the information (please specify): </li> </ul>	ne applicant/con n report. lineation report ueson & Tueson SV il Survey. Citati Metro, DigitalGlobe el photographs, Jun onse letter:	v ion: ne 29th, 2017 	Id not be relied upon for later jurisdictional determinations. i2/5/7017	
Signature and Date of Regulatory Project Manager (REQUIRED)			Person Requesting Preliminary JD obtaining the signature is impracticable)	
EXPLANATION OF PRELIMINARY AND APPROVED JURISDICTIONAL D 1. The Corps of Engineers believes that there may be jurisdictional waters of the Un- hereby advised of his or her option to request and obtain an approved jurisdictional d has declined to exercise the option to obtain an approved JD in this instance and at thi 2. In any circumstance where a permit applicant obtains an individual permit, or a Na or requests verification for a non-reporting NWP or other general permit, and the pe following; (1) the permit applicant has elected to seek a permit authorization based of the option to request an approved JD before accepting the terms and conditions of compensatory mitigation being required or different special conditions. (3) that the a other general permit authorization; (4) that the applicant can accept a permit authorization acceptance of the use of the preliminary JD, but that either form of JD will be proc undertaking any activity in reliance on any form of Corps permit authorization based that activity are jurisdictional waters of the United States, and precludes any chalter appeal or in any Federal court; and (7) whether the applicant elects to use either an proffered individual permit (and all terms and conditions contained therein), or indiv- appeal, jurisdictional issues can be raised (see 33 C F R, 331.5(ar(2)) 1f, during that site, or to provide an Oficial delineation of jurisdictional waters on the site. the Corms	nited States on the sul determination (JD) fo- is time. attionwide General Pe- ermit applicant has n on a preliminary JD, y of the permit authoriz applicant has the righ trization and thereby a tivity in reliance upor cessed as soon as is j cosed as soon as is j cosed as soon as is j to a preliminary JD nge to such jurisdicti n approved JD or a p vidual permit denial administrative appeal	bject site, and the p i that site. Neverthis irmit (NWP) or otho ot requested an app which does not mad- ation, and that bas it to request an ind ingree to comply wi a the subject permi- practicable; (6) acc constitutes agreents on in any administ oreliminary JD, tha- can be administrati- i, it becomes necessi	eless, the permit applicant or other person who requested this preliminary JE er general permit verification requiring "preconstruction notification" (PCN) proved JD for the activity, the permit applicant is hereby made aware of the ice an official determination of jurisdictional waters: (2) that the applicant has ing a permit authorization on an approved JD could possibly result in less violal permit rather than accepting the terms and conditions of the NWP of the latter than accepting the terms and conditions of the NWP of thall the terms and conditions of that permit, including whatever mitigation a turborization without requesting an approved JD constitutes the applicant" epting a permit authorization (e.g., signing a proffered individual permit) or ent that all wetlands and other water bodies on the site affected in any way by rative or judicial compliance or enforcement action, or in any administrative UD will be processed as soon as is practicable. Further, an approved JD, vely appealed pursuant to 33 C F R. Part 331, and that in any administrative ary to make an official determination whether CWA jurisdiction exists over	

THIS PAGE INTENTIONALLY LEFT BLANK

# STORMWATER DRAINAGE PLAN

# THIS PAGE INTENTIONALLY LEFT BLANK



# STORMWATER DRAINAGE PLAN PROPOSED AIRFIELD SAFETY ENHANCEMENT PROJECT ENVIRONMENTAL IMPACT STATEMENT

TUCSON INTERNATIONAL AIRPORT TUCSON, PIMA COUNTY, ARIZONA

> **Prepared by:** TYLIN International 60 East Rio Salado Parkway, Suite 501 Tempe, Arizona 85281

> > January 2018

**TYLININTERNATIONAL** engineers | planners | scientists



Та	ble of Contents	
1.0		3
	1.1 Purpose of Report	4
	1.2 Location	4
2.0	) Tucson International Airport	5
	2.1 Existing Conditions	5
	2.2 Previous Studies	7
3.0	) Existing Conditions Hydrology	8
	3.1 Existing Conditions	8
	3.2 Previous Study Model Results	8
	3.3 Airfield Wash Hydrology	9
4.0	) Hydrology	10
4	4.1 Design Criteria	10
4	4.2 Proposed Airfield Improvements	10
4	4.3 Proposed Onsite Hydrologic Conditions	11
5.0	) Hydraulics	13
ļ	5.1 Proposed Drainage Improvements	13
į	5.2 Pipe Culverts	13
į	5.3 Channels	14
ţ	5.4 Detention Basins	15
6	References	16

# List of Figures

Figure-1: Proposed Airfield Improvements	.4
Figure-2: Location Map	.5
Figure-3: Offsite Drainage Flow Paths	.6



# Table of Contents (continued)

# List of Tables

Table-3.2:Airfield Discharges	8
Table 3.3 – Airfield Watershed Existing Conditions Subbasin Discharge	9
Table 4.3a – Airfield Watershed Proposed Conditions Subbasin Discharge	11
Table 4.3b – Pre vs. Post Discharges	12
Table 4.3c – Change in Discharges	12
Table 5.2 – Culvert Summary	14

# List of Appendices

### Appendix A – Onsite Calculations

- Appendix A.1 NOAA Atlas 14
- Appendix A.2 Calculations

## Appendix B – Basin Sizing

# Appendix C – Exhibits

- Exhibit 1 Soils Map Existing Conditions
- Exhibit 2 Soils Map Proposed Conditions
- Exhibit 3 Land Use Existing Conditions
- Exhibit 4 Land Use Proposed Conditions
- Exhibit 5 Existing Conditions Work Map
- Exhibit 6 Proposed Conditions Work Map
- Exhibit 7 Proposed Conditions Basin Exhibit

Appendix D – FlowMaster (Pipe Sizing)



# 1.0 Introduction

The Federal Aviation Administration (FAA) issued a *Federal Register* Notice on August 19, 2016, announcing its intent to prepare an Environmental Impact Statement (EIS) for the Proposed Airfield Safety Enhancement Project (ASEP) including real property transactions at Tucson International Airport (TUS or Airport) in Pima County, Arizona (the Proposed Action).

The FAA is the lead federal agency for preparation of the EIS and will do so in compliance with National Environmental Policy Act of 1969 (NEPA) and Council on Environmental Quality (CEQ) Regulations for Implementing the Procedural Provisions of NEPA (40 Code of Federal Regulations [CFR] Parts 1500-1508), as well as FAA's policies and procedures for complying with NEPA found in FAA Order 1050.1F, *Environmental Impacts: Policies and Procedures* and FAA Order 5050.4B, *NEPA Implementing Instructions for Airport Actions*. The FAA has invited the United States Air Force (USAF) and the National Guard Bureau (NGB) to participate as cooperating agencies as described under 40 CFR § 1501.6 and both have accepted FAA's invitation.

The Proposed Action includes the construction of a new air carrier runway parallel to the primary Runway 11L/29R. This new runway would replace the existing general aviation Runway 11R/29L. The purpose of the project is to enhance the safety of the airfield by eliminating areas in which risk of runway collision and incursion are heightened. Construction of an additional runway will simplify the current airfield's complex geometry, thus, enhancing the overall safety of the runway and its operations.

The key project elements include the following:

- Relocate Runway 11R/29L to the southwest and construct it to a total length of 10,996 feet and width of 150 feet
- Construct new full-length parallel taxiway between Runway 11L/29R and Runway 11R/29L
- Construct supporting connector taxiways between Runway 11R/29L and both outboard and centerline parallel taxiways
- Construct bypass taxiways for Runways 11L and 11R
- Closure of segments of taxiway A2 between taxiway A and Runway 3/21 and taxiway A2 and Runway 3/21
- Construct/maintain Arizona Air National Guard (AANG) extended blast pads for Runways 11L/29R and 11R/29L
- Construction of additional drainage detention areas to support additional impervious pavement areas
- Construction of replacement Earth Covered Magazines on U.S. Air Force Plant 44 (AFP 44)
- Construction of a Munitions Storage Area on land identified as "Parcel H" by the National Guard Bureau



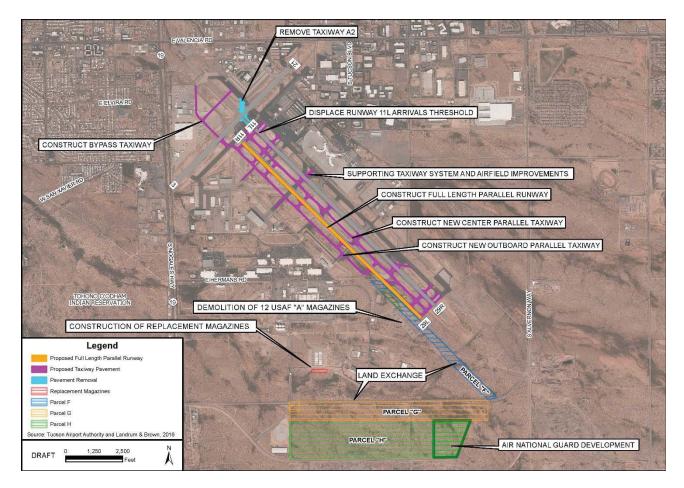


Figure-1: Proposed Airfield Improvements

This stormwater drainage plan is to be used by the FAA to document the conceptual design with recommendations of drainage improvements including conveyance facilities and detention basins to mitigate increases in runoff discharge and volumes associated with the Proposed Action.

### 1.1 Purpose of Report

This report focuses on development of conceptual drainage improvements in support of the Proposed Action. The report evaluates existing hydrologic conditions and develops a conceptual plan for stormwater management including an evaluation of pre versus post runoff conditions at offsite discharge locations. This report documents the conceptual design with recommendations of drainage improvements including conveyance facilities and detention basins to mitigate increases in runoff discharge and volumes.

### 1.2 Location

The Airport is located on 8,343 acres in Tucson, Arizona in Pima County south of the city of Tucson central business district. The Airport is near both Interstate 10 and Interstate 19. The United States Air Force (USAF) owned land, known as Air Force Plant 44 (AFP 44), is located along the southwest border of the Airport.



The Airport is bounded by Valencia Road (north), Alvernon Way (east), Aerospace Parkway (south) and Nogales Highway (west) within the city of Tucson, Arizona.

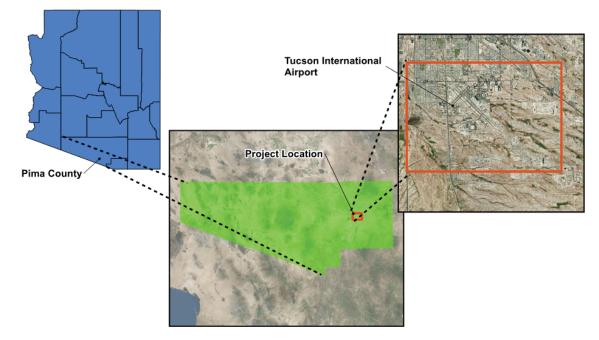


Figure-2: Location Map

# 2.0 <u>Tucson International Airport</u>

# 2.1 Existing Conditions

The TUS airfield is comprised of three runways; one set of close parallel runways separated by a distance of 706 feet (oriented in a northwest/southeast direction) and one crosswind runway (oriented in a northeast/southwest direction).

Parallel Runways 11L/29R and 11R/29L measure 10,996 feet long by 150 feet wide and 8,408-feet long by 75-feet wide, respectively. The crosswind runway, Runway 3/21, measures 7,000 feet long by 150-feet wide.

Runway 11L/29R is the primary runway at TUS and is the runway generally used by air carrier and military aircraft. During adverse wind conditions, air carrier and military aircraft occasionally use crosswind Runway 3/21. The crosswind runway is also used for convenience by General Aviation (GA) aircraft when conditions allow. Runway 11R/29L, originally built as a taxiway, has been converted to a runway primarily used by GA aircraft, due to its length and width.

The taxiway system provides aircraft access between the runways and the passenger terminal complex, general and corporate aviation areas, military facilities, airfreight terminals, and other aircraft parking areas.



There are five major drainages close to the Airport, Airport Wash, Valencia Wash, El Vado Wash, Santa Clara Wash, and Hughes Wash, all of which are part of the larger Santa Cruz River watershed. These washes are considered ephemeral streams because they only conduct water during and immediately following precipitation events. Perennial streams conduct water all year long and intermittent streams are dry for part of the year, but conduct water for periods longer than ephemeral streams. During a precipitation event, stormwater runoff from the Airport is conveyed by a system of manmade channels and culverts to these drainages, which flow from southeast to northwest toward the Santa Cruz River.

Airport Wash concentrates on the northeast side of the 11L/29R and the terminal area is conveyed around TUS via the Airport Wash channel, which ultimately discharges north of Valencia Road east of Park Avenue. Hughes Wash conveys flow to the southwest side of AFP 44 and ultimately discharges west of Nogales Highway south of Hermans Road.

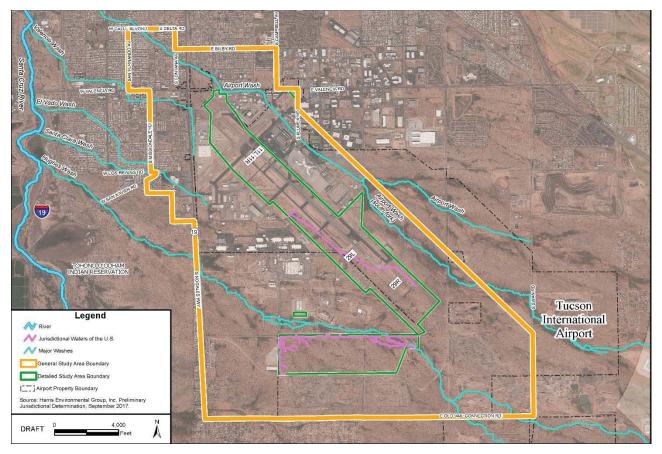


Figure-3: Offsite Drainage Flow Paths

Within the airfield, a smaller local watershed identified as the Airfield watershed collects and conveys onsite runoff from existing airport facilities and currently conveys and discharges stormwater runoff at a number of local outfalls located adjacent to the Nogales



Highway. Three existing culverted crossings of the railroad and the highway, between Valencia Road and Hermans Road, discharge flow to the west side of the Nogales Highway where the stormwater is typically conveyed within existing natural washes to the northwest toward the Santa Cruz River.

There are four areas where ponding may occur at the Airport during heavy rain events. These are: 1) within the airfield, 2) the area west of Bombardier Aerospace at the railroad, 3) the area west of the Triple Hangars at the railroad, and 4) within Airport Wash. Ponding on the airfield occurs between all runways and taxiways. The ponding is temporary in nature, and only occurs in a significant amount during storms with frequencies greater than 10 years.

## 2.2 Previous Studies

The following previous studies have developed existing conditions hydrology and hydraulics for the Airport area and were reviewed as part of this effort.

- Airport Wide Drainage Basin Study (AWDBS). May 1992.
- Draft Airport Wide Drainage Basin Update. Stantec Consulting, Inc. August, 2004. This report is an update of the original 1992 report to incorporate changes in land use, current agency requirements and revised Master Plan conditions.



# 3.0 Existing Conditions Hydrology

Offsite hydrologic analysis was not performed with this study. Instead, the results of a previous study (*Airport Wide Drainage Update Final Report* (Report), prepared by Stantec Consulting, Inc. 2004) was used. This report refers to some data from the *Airport Wide Drainage Basin Study* (AWDBS) completed in May 1992, while providing additional modeling results and updated analysis.

In the 2004 study, U.S. Army Corps of Engineers (USACE) HEC-1, Flood Hydrograph Package computer program was used to determine the stormwater runoff discharges of the existing airport conditions for the 2, 5, 10 and 100-year return periods. At the time, the HEC-1 model was used in place of the standard Pima County hydrology methodology because of the nature of the contributing watersheds. The HEC-1 model was used to account for the extensive ponding throughout the watershed, which largely affects the peak discharges.

## 3.1 Existing Conditions

Per the 2004 report, several watersheds contribute to the study area or adjacent surrounding areas. These watersheds are the Airport Wash, Hughes Wash and the Airfield Wash watersheds.

# 3.2 Previous Study Model Results

The existing airfield drainage facilities have generally been designed in accordance with the FAA guidelines. FAA Advisory Circular AC 150/5320-5B, "Airport Drainage," July 1970 recommends that airfield drainage facilities be designed for the 5-year frequency storm runoff. Per the results, the peak 5-year baseline flows from the site is listed as 222 cubic feet per second (cfs). The detention basin volume shown in Table 3.2 is indicated as future development within the Stantec report.

	r			
	Storm	Baseline	Post-Development	Detention
	Frequency	Flow	Flow	Basin Volume
		[cfs]	[cfs]	[acft.]
Airfield Watershed (Point C)	2	108	152	4
	5	222	305	4
	10	322	379	5
	100	904	981	5

Notes: 1) Hydrology modeled using HEC-1 hydrographs and stage-storage-discharge relationships.

2) Uncertain if detention basins were constructed. Listed as 'future' development in the Stantec report (2004).

3) Results in table based upon 'on-line' detention basins in Airfield Wash and 20% oversizing.



# 3.3 Airfield Wash Hydrology

There are six distinct stormwater outfalls from the Airfield Wash watershed (see Exhibit 5 in Appendix C). Each of these outfalls has a distinct drainage area contributing stormwater runoff. These six subbasins of Airfield wash are analyzed to determine the peak discharges reaching each outfall. The City of Tucson's hydrologic method was used to develop onsite discharges with the following results.

Drainage	Outfall	Contributing	Weighted	5-year	100-year
Areas	Location	Area	Runoff C	Discharge	Discharge
		[acres]		[cfs]	[cfs]
1	Valencia Road to Airport Wash	41.8	0.73	27.4	78.4
2	Nogales Hwy to El Vado Wash	160.9	0.85	95.1	271.6
3	Nogales Hwy to Santa Clara Wash	77.3	0.80	40.2	115.0
4	Nogales Hwy to Santa Clara Wash	618.7	0.78	165.9	474.0
5	Hermans Road to Hughes Wash	593.3	0.77	119.5	341.3
6	Hermans Road to Hughes Wash	64.8	0.86	40.2	114.9

Table 3.3 – Airfield Watershed Existing Conditions Subbasin Discharge

The results of the existing conditions analysis determine the base flow rate which are not to be exceeded by proposed conditions in the Proposed Action.



## 4.0 <u>Hydrology</u>

## 4.1 Design Criteria

Section 1.5 of the AWDB-Update designates that future drainage facilities be designed in accordance with the following City of Tucson, Pima County and FAA guidelines:

- Detention basins will hold runoff for a period of time before releasing it to downstream facilities, and must drain within 24-hours per Pima County DOT & Flood Control District (FCD) regulations. <u>The basins will be designed such that</u> <u>post-development 2, 5, 10 & 100-year peak flows from the site will not exceed the</u> <u>predevelopment values</u>.
- Detention volumes in onsite ponding areas and detention basins will bleed-off flow such that the basins will drain within 24-hours.
- Per FAA guidelines, <u>future onsite drainage facilities mush have capacity for the 5-year frequency storm runoff</u>. Additionally, temporary ponding from storms with a return period of 10-years will be checked for encroachment into the runway and taxiway safety areas. Ponding in the airfield is allowed only as a result of runoff exceeding the 5-year design capacity. Detention basins within the runway and taxiways safety areas will not be allowed. Temporary or short term ponding in the airfield caused by runoff from rainfall events greater than the 5-year event must drain within 24-hours.
- Detention basins shall be located as far from runways as possible.
- Buildings, structures and adjacent facilities shall be protected from the 100-year frequency storm runoff.
- No changes in drainage patterns impacting downstream areas will be allowed.

## 4.2 Proposed Airfield Improvements

The Proposed Action includes construction of a full length parallel runway designated 11R/29L, a new center parallel taxiway, new outboard parallel taxiway, addition of supporting and bypass taxiway systems (see Figure 1). These improvements are entirely located within the Airfield Wash watershed and constitute an overall increase in the total impervious area located within the watershed resulting in a net increase in stormwater runoff discharge and volume.

The nature of the improvements can be observed by comparing the existing onsite development shown in Exhibit 5 with the proposed shown in Exhibit 6 in Appendix C. The change in land use can be classified into three categories:

- Impervious which is now pervious, resulting from the removal of an impervious surface
- Pervious which is now impervious, resulting from the addition of new impervious surfaces; and
- Impervious which will remain impervious, resulting from a modification in the Proposed Action but from one impervious surface to another.



The net increase in impervious surface is approximately 80 acres which is primarily split between subbasins 4 and 5. Subbasins 1, 2 and 6 were essentially unchanged while subbasin 3, although modified, resulted in a zero-net change in impervious surface.

## 4.3 Proposed Onsite Hydrologic Conditions

The six subbasins which make up the Airfield Wash watershed have been modified to reflect physical changes to the existing conditions. The proposed conditions drainage boundaries are adjusted to account for changes in contributing watershed based upon the runway and taxiway configuration (see Exhibit 6 in Appendix C).

The drainage analysis follows the guidelines within the *Standards Manual for Drainage Design and Floodplain Management in Tucson, Arizona, July 1998* (Tucson Drainage Manual). A base rainfall intensity found in the Tucson Drainage Manual was used to calculate 100-year discharges. Other storm frequencies were determined using a factor found in Table 4.5. This methodology applies a weighted runoff coefficient by soils and land use categories, a rainfall intensity based upon NOAA 14 rainfall data and the contributing area. The following results are documented in the calculations:

Drainage	Outfall	Contributing	Weighted	5-year	100-year
Areas	Location	Area	Runoff C	Discharge	Discharge
		[acres]		[cfs]	[cfs]
1	Valencia Road to Airport Wash	41.8	0.73	27.4	78.4
2	Nogales Hwy to El Vado Wash	160.9	0.85	95.1	271.6
3	Nogales Hwy to Santa Clara Wash	77.3	0.80	40.2	115.0
4	Nogales Hwy to Santa Clara Wash	588.8	0.83	173.3	495.0
5	Hermans Road to Hughes Wash	623.1	0.81	135.6	387.3
6	Hermans Road to Hughes Wash	64.8	0.86	40.2	114.9

Table 1 2a	Airfield Watershee	Proposed Cond	itions Subbasin D	lischargo
1 anie 4.3a –	AIIIICIU Walcisiicu	i Fioposeu Conui	illonis Subbasin D	ischarge

It is important to note that the rational methodology in the Tucson Drainage Manual does not account for retention/detention. Retention/Detention is handled external to the runoff calculations.

The net change in impervious area is calculated and reported as approximately 80 acres. In order to attenuate the increase in stormwater runoff (both discharge and volume) due to the Proposed Action, stormwater storage is needed within the Airfield watershed to attenuate both the discharge and the volume of runoff released from the watershed. Table 4.3b summarizes the change in 5 and d100-year discharges.



		Existing Co	nditions		Proposed Conditions				
Drainage	Contributing	Weighted	5-year	100-year	Contributing	Weighted	5-year	100-year	
Areas	Area	Runoff C	Discharge	Discharge	Area	Runoff C	Discharge	Discharge	
	[acres]		[cfs]	[cfs]	[acres]		[cfs]	[cfs]	
1	41.8	0.73	27.4	78.4	41.8	0.73	27.4	78.4	
2	160.9	0.85	95.1	271.6	160.9	0.85	95.1	271.6	
3	77.3	0.8	40.2	115.0	77.3	0.8	40.2	115.0	
4	618.7	0.78	165.9	474.0	588.8	0.83	173.3	495.0	
5	593.3	0.77	119.5	341.3	623.1	0.81	135.6	387.3	
6	64.8	0.85	40.2	114.9	64.8	0.86	40.2	114.9	

Subbasins 1, 2, 3 and 6 have no appreciable increase in stormwater runoff. However, subbasins 4 and 5 increase for both the 5 and 100-year discharge.

	Existing C	onditions	Proposed	Conditions		
Drainage	5-year	100-year	5-year	5-year 100-year		100-year
Areas	Discharge	Discharge	Discharge	Discharge	change	change
	[cfs]	[cfs]	[cfs]	[cfs]	[cfs]	[cfs]
1	27.4	78.4	27.4	78.4	0.0	0.0
2	95.1	271.6	95.1	271.6	0.0	0.0
3	40.2	115	40.2	115.0	0.0	0.0
4	165.9	474	173.3	495.0	7.4	21.0
5	119.5	341.3	135.6	387.3	16.1	46.0
6	40.2	114.9	40.2	114.9	0.0	0.0

Table 4.3c –	Change	in Discharges	S
--------------	--------	---------------	---



## 5.0 Hydraulics

## 5.1 Proposed Drainage Improvements

In many respects the airfield drainage after the implementation of the Proposed Action will be similar to existing conditions in that stormwater will still collect in the infield areas between the runways and taxiways. However, the collection system to convey the stormwater away from the airfield will, by necessity, be revised to meet the needs of the airfield improvements.

There are currently two outfalls for stormwater runoff within the airfield. These are:

- 1. An existing channel located approximately mid-field near Aero Park Boulevard conveys stormwater runoff southwesterly and discharges to a retention/detention area on the south side of Hermans Road adjacent to Nogales Highway. This discharges to Hughes Wash (Subbasin 5).
- 2. An existing channel located near the norther end of the airfield, south of and nearly adjacent to runway 3/21. This channel conveys flow westerly to an existing crossing of Nogales Highway located approximately 1500-feet south of Los Reales Road (Subbasin 4).

The proposed drainage concept (see Exhibit 6 in Appendix C) connects the infield areas between the runways and taxiways using culverted crossings and discharge to the two existing conveyance channels. New in-line detention facilities would be located within open/available spaces to mitigate discharges to acceptable pre-project rates to meet drainage design guidelines.

## 5.2 Pipe Culverts

In order to accommodate and effectively convey the onsite flows through the infield areas of the airfield, pipe culverts would be required to route stormwater through the infield areas. The size, length, and dimensions of the pipe are determined based upon the conveyance of the accumulated 5-year discharge reaching each culvert. The pipe material would be determined based upon available cover and airport loading over the top of the pipe. It is recommended that Class V rubber gasket reinforced concrete pipe (RGRCP), or a suitable material able to withstand aircraft loading, be used with a minimum of 3-feet cover.

Local onsite hydrology methods were used to determine the discharge based upon an accumulating contributing watershed and a lengthening time of concentration. These local discharges determine the required conveyance capacity for culverts located within subbasins 4 and 5 (see table 5.2).



Subbasin	Culvert	5-year	Culvert	Subbasin	Culvert	5-year	Culvert
ID	ID	Discharge	Diameter	ID	ID	Discharge	Diameter
		[cfs]	[inches]			[cfs]	[inches]
4	C-1	7.8	18	5	C-16	6.2	18
4	C-2	9.7	24	5	C-17	12.1	24
4	C-3	15.3	24	5	C-18	8.2	18
4	C-4	18.7	24	5	C-19	14.6	24
4	C-5	21.8	30	5	C-20	27.1	30
4	C-6	8.1	18	5	C-21	29.1	30
4	C-7	8.8	18	5	C-22	34.3	30
4	C-8	18.7	30	5	C-23	14.2	24
4	C-9	13.5	24	5	C-24	12	24
4	C-10	17.1	24	5	C-25	27.2	30
4	C-11	19.7	24	5	C-26	0.2	24
4	C-12	11.8	24	5	C-27	58.1	42
4	C-13	20.7	24	-	-	-	-
4	C-14	31.1	30	-	-	-	-
4	C-15	62.9	42	-	-	-	-

Per the drainage design guidelines in Section 4.1, culverts shall have, at a minimum, the ability to convey the 5-year discharge. The 10-year is allowed to temporarily pond as long as stormwater does not pond into the runway or taxiways. This is an important distinction and special care should be taken during final design to ensure that the culverts are sized properly to meet both criteria.

Culverts can also become blocked due to debris, so regular maintenance should be performed. A minimum pipe diameter should be considered (recommend at least 24-inches in diameter) so that the culverts are less susceptible to debris blockage. Upsizing the culvert diameter a half size (6-inches) should also be considered if regular maintenance is problematic.

## 5.3 Channels

The existing channels identified in Section 5.1 have been evaluated for capacity based upon a rough estimate of top and bottom width, sideslope, depth and longitudinal slope. The channel segments may need to be enlarged depending upon existing capacity, proposed conveyance, and detention basin location.

Based upon existing conditions, it is estimated that the existing channel network has capacity for between a 5 and 10-year storm event based upon physical location, dimensions depth and longitudinal slope. Stormwater runoff in excess of the capacity of



the channel would sheet flow generally following the slope of the terrain and between built up areas.

In some areas the channel is relatively clean with a consistent trapezoidal shape. In other areas the channels are roughly graded with varying levels of vegetation. The capacity of the existing channel could be improved through maintenance by removing dense sections of vegetation in multiple reaches. Channel capacity could also be improved by consistent grading and dimensioning of the channel shape and slope.

## 5.4 Detention Basins

The City of Tucson's method for developing pre and post conditions hydrographs was used to determine the required storage volume necessary to attenuate the peak runoff discharge to that of existing conditions. Calculations are provided in Appendix A and B.

The net change in impervious area was determined through review of changes in the Land Use as described in Section 4.2. Other adjustments include a change in the contributing drainage area within each subbasin based upon modifications to the flow patterns within the runway and taxiway areas. The resulting hydrographs were reviewed to determine the additional volume (as the area under the curve) between the existing and proposed conditions runoff.

The storage volume necessary to attenuate the 100-year onsite flows due to the Proposed Action were calculated to be **11.6 acre-feet** split between subbasins 4 and 5 as 0.8 acre-feet and 10.8 acre-feet respectively. The storage volume provided by the proposed detention basins would be up to **31.1 acre-feet** split between subbasins 4 and 5 as 20.1 acre-feet and 11.0 acre-feet respectively.

Therefore, the three proposed detention basins (see Exhibit 7) would effectively attenuate the stormwater discharge and volume as a result of the Proposed Action.

The detention basins would be designed with a positive slope to the outfall, which will allow for release of detained flow such that the basins will discharge all runoff within a 24hour period. Adjustments to size, shape, and location of the proposed detention basins would be made during final design to avoid utilities or other existing or planned obstacles. However, it is important to maintain the connectivity between the collection and delivery channels /pipes which bring stormwater to the basins, and then from the basins downstream conveyance to the outfalls.

The FAA has confirmed the preliminary layout of the basins would be acceptable as long as they are clear of the appropriate runway safety areas (ROFA, RSA, etc.). Specifically, FAA must ensure the basins are out the new south parallel taxiway safety area and the Obstacle Free Area (OFA).





## 6 <u>References</u>

- 1. Airport Wide Drainage Basin Study (AWDBS). May 1992.
- 2. Draft Airport Wide Drainage Basin Update. Stantec Consulting, Inc. August, 2004.
- 3. Standards Manual for Drainage Design and Floodplain Management in Tucson, Arizona. City of Tucson. December 1989.
- 4. Stormwater Detention/Retention Manual. Pima County Department of Transportation and Flood Control.
- 5. Airport Drainage, Federal Aviation Authority AC 150/5320-5B. July 1970.

Tucson International Airport

Conceptual Drainage Design



# Appendix A – Onsite Calculations

Tucson International Airport

Conceptual Drainage Design



# Appendix A.1 – NOAA Atlas 14



NOAA Atlas 14, Volume 1, Version 5 Location name: Tucson, Arizona, USA\* Latitude: 32.1192°, Longitude: -110.9428° Elevation: 2592.28 ft\*\* \* source: ESRI Maps \*\* source: USGS



#### POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

PF\_tabular | PF\_graphical | Maps\_&\_aerials

## PF tabular

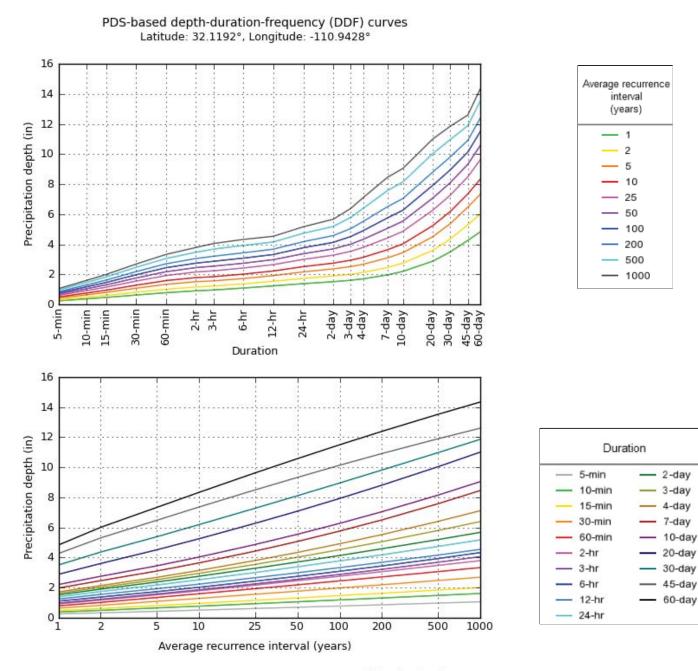
PDS	-based po	oint precij	oitation fr			with 90%		ce interva	ls (in inc	hes) <sup>1</sup>
Duration				Averag	e recurrenc	e interval (y	ears)			
Buration	1	2	5	10	25	50	100	200	500	1000
5-min	<b>0.252</b>	<b>0.324</b>	<b>0.428</b>	<b>0.507</b>	<b>0.614</b>	<b>0.695</b>	<b>0.779</b>	<b>0.862</b>	<b>0.974</b>	<b>1.06</b>
	(0.226-0.284)	(0.291-0.366)	(0.382-0.481)	(0.449-0.569)	(0.537-0.687)	(0.599-0.779)	(0.661-0.877)	(0.720-0.975)	(0.792-1.11)	(0.844-1.22)
10-min	<b>0.383</b>	<b>0.493</b>	<b>0.651</b>	<b>0.772</b>	<b>0.935</b>	<b>1.06</b>	<b>1.19</b>	<b>1.31</b>	<b>1.48</b>	<b>1.61</b>
	(0.344-0.433)	(0.443-0.558)	(0.580-0.732)	(0.682-0.866)	(0.816-1.05)	(0.912-1.19)	(1.01-1.34)	(1.10-1.49)	(1.21-1.69)	(1.28-1.85)
15-min	<b>0.475</b>	<b>0.611</b>	<b>0.808</b>	<b>0.956</b>	<b>1.16</b>	<b>1.31</b>	<b>1.47</b>	<b>1.63</b>	<b>1.84</b>	<b>2.00</b>
	(0.426-0.536)	(0.549-0.692)	(0.719-0.908)	(0.846-1.07)	(1.01-1.30)	(1.13-1.47)	(1.25-1.66)	(1.36-1.84)	(1.49-2.10)	(1.59-2.30)
30-min	<b>0.639</b>	<b>0.823</b>	<b>1.09</b>	<b>1.29</b>	<b>1.56</b>	<b>1.77</b>	<b>1.98</b>	<b>2.19</b>	<b>2.48</b>	<b>2.69</b>
	(0.574-0.722)	(0.739-0.931)	(0.969-1.22)	(1.14-1.45)	(1.36-1.74)	(1.52-1.98)	(1.68-2.23)	(1.83-2.48)	(2.01-2.82)	(2.14-3.09)
60-min	<b>0.791</b> (0.710-0.894)	<b>1.02</b> (0.914-1.15)	<b>1.35</b> (1.20-1.51)	<b>1.59</b> (1.41-1.79)	<b>1.93</b> (1.69-2.16)	<b>2.19</b> (1.88-2.45)	<b>2.45</b> (2.08-2.76)	<b>2.71</b> (2.26-3.07)	<b>3.06</b> (2.49-3.50)	<b>3.33</b> (2.65-3.83)
2-hr	<b>0.917</b>	<b>1.17</b>	<b>1.52</b>	<b>1.79</b>	<b>2.17</b>	<b>2.46</b>	<b>2.76</b>	<b>3.06</b>	<b>3.48</b>	<b>3.79</b>
	(0.827-1.03)	(1.06-1.32)	(1.37-1.70)	(1.59-2.00)	(1.91-2.41)	(2.14-2.73)	(2.36-3.07)	(2.57-3.42)	(2.84-3.92)	(3.04-4.32)
3-hr	<b>0.973</b> (0.877-1.09)	<b>1.23</b> (1.11-1.38)	<b>1.58</b> (1.42-1.77)	<b>1.86</b> (1.66-2.08)	<b>2.25</b> (1.98-2.50)	<b>2.55</b> (2.22-2.84)	<b>2.88</b> (2.45-3.22)	<b>3.21</b> (2.68-3.61)	<b>3.69</b> (2.98-4.19)	<b>4.06</b> (3.20-4.66)
6-hr	<b>1.10</b> (0.993-1.24)	<b>1.38</b> (1.24-1.55)	<b>1.73</b> (1.55-1.94)	<b>2.03</b> (1.80-2.26)	<b>2.43</b> (2.13-2.71)	<b>2.75</b> (2.38-3.07)	<b>3.09</b> (2.63-3.45)	<b>3.44</b> (2.88-3.86)	<b>3.93</b> (3.19-4.45)	<b>4.33</b> (3.45-4.95)
12-hr	<b>1.24</b>	<b>1.55</b>	<b>1.93</b>	<b>2.23</b>	<b>2.66</b>	<b>2.99</b>	<b>3.33</b>	<b>3.68</b>	<b>4.16</b>	<b>4.54</b>
	(1.12-1.38)	(1.41-1.73)	(1.73-2.15)	(2.00-2.48)	(2.35-2.95)	(2.61-3.33)	(2.86-3.72)	(3.11-4.14)	(3.43-4.72)	(3.67-5.20)
24-hr	<b>1.39</b>	<b>1.74</b>	<b>2.17</b>	<b>2.52</b>	<b>3.00</b>	<b>3.38</b>	<b>3.78</b>	<b>4.18</b>	<b>4.74</b>	<b>5.17</b>
	(1.28-1.53)	(1.60-1.91)	(1.99-2.38)	(2.30-2.77)	(2.72-3.30)	(3.04-3.72)	(3.36-4.18)	(3.68-4.65)	(4.11-5.33)	(4.43-5.86)
2-day	<b>1.52</b> (1.40-1.67)	<b>1.90</b> (1.75-2.09)	<b>2.37</b> (2.18-2.60)	<b>2.76</b> (2.52-3.02)	<b>3.28</b> (2.99-3.60)	<b>3.70</b> (3.33-4.07)	<b>4.14</b> (3.69-4.57)	<b>4.58</b> (4.05-5.10)	<b>5.19</b> (4.51-5.85)	<b>5.67</b> (4.85-6.46)
3-day	<b>1.62</b>	<b>2.02</b>	<b>2.53</b>	<b>2.95</b>	<b>3.54</b>	<b>4.02</b>	<b>4.52</b>	<b>5.05</b>	<b>5.79</b>	<b>6.39</b>
	(1.49-1.77)	(1.86-2.22)	(2.32-2.77)	(2.69-3.23)	(3.21-3.88)	(3.61-4.42)	(4.02-5.01)	(4.43-5.63)	(4.97-6.53)	(5.39-7.28)
4-day	<b>1.71</b>	<b>2.14</b>	<b>2.68</b>	<b>3.14</b>	<b>3.80</b>	<b>4.34</b>	<b>4.91</b>	<b>5.52</b>	<b>6.40</b>	<b>7.11</b>
	(1.58-1.88)	(1.96-2.35)	(2.45-2.94)	(2.86-3.44)	(3.43-4.17)	(3.88-4.78)	(4.34-5.44)	(4.81-6.16)	(5.43-7.22)	(5.92-8.11)
7-day	<b>1.97</b>	<b>2.46</b>	<b>3.10</b>	<b>3.64</b>	<b>4.42</b>	<b>5.06</b>	<b>5.76</b>	<b>6.50</b>	<b>7.57</b>	<b>8.45</b>
	(1.81-2.17)	(2.25-2.71)	(2.83-3.41)	(3.31-4.00)	(3.98-4.87)	(4.51-5.60)	(5.07-6.41)	(5.64-7.30)	(6.42-8.62)	(7.04-9.74)
10-day	<b>2.21</b>	<b>2.75</b>	<b>3.45</b>	<b>4.03</b>	<b>4.86</b>	<b>5.54</b>	<b>6.27</b>	<b>7.04</b>	<b>8.14</b>	<b>9.04</b>
	(2.02-2.42)	(2.52-3.02)	(3.14-3.78)	(3.67-4.42)	(4.38-5.34)	(4.94-6.11)	(5.52-6.95)	(6.11-7.88)	(6.90-9.22)	(7.52-10.4)
20-day	<b>2.88</b> (2.65-3.15)	<b>3.60</b> (3.30-3.94)	<b>4.51</b> (4.12-4.93)	<b>5.25</b> (4.78-5.73)	<b>6.27</b> (5.67-6.85)	<b>7.08</b> (6.34-7.76)	<b>7.92</b> (7.02-8.73)	<b>8.81</b> (7.71-9.78)	<b>10.0</b> (8.61-11.3)	<b>11.0</b> (9.29-12.5)
30-day	<b>3.50</b> (3.24-3.80)	<b>4.36</b> (4.03-4.74)	<b>5.38</b> (4.96-5.84)	<b>6.19</b> (5.69-6.71)	<b>7.27</b> (6.64-7.90)	<b>8.10</b> (7.36-8.83)	<b>8.95</b> (8.07-9.80)	<b>9.81</b> (8.76-10.8)	<b>11.0</b> (9.64-12.2)	<b>11.9</b> (10.3-13.3)
45-day	<b>4.26</b> (3.95-4.61)	<b>5.30</b> (4.92-5.74)	<b>6.47</b> (6.00-7.00)	<b>7.35</b> (6.81-7.96)	<b>8.49</b> (7.83-9.20)	<b>9.32</b> (8.55-10.1)	<b>10.1</b> (9.25-11.0)	<b>10.9</b> (9.89-11.9)	<b>11.9</b> (10.7-13.1)	<b>12.6</b> (11.2-14.0)
60-day	<b>4.83</b> (4.47-5.22)	<b>6.01</b> (5.56-6.51)	<b>7.33</b> (6.78-7.93)	<b>8.33</b> (7.69-9.01)	<b>9.62</b> (8.85-10.4)	<b>10.6</b> (9.69-11.5)	<b>11.5</b> (10.5-12.5)	<b>12.4</b> (11.2-13.6)	<b>13.5</b> (12.1-15.0)	<b>14.3</b> (12.7-16.0)

<sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

Back to Top

## **PF graphical**



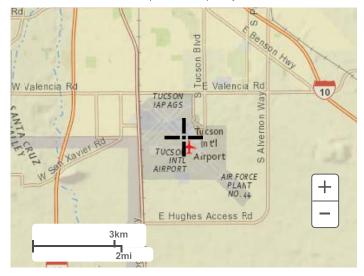
NOAA Atlas 14, Volume 1, Version 5

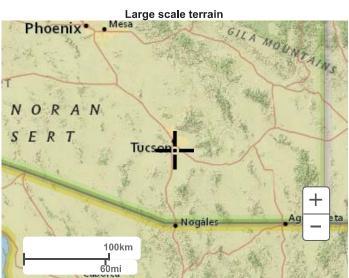
Created (GMT): Thu Jul 20 16:18:00 2017

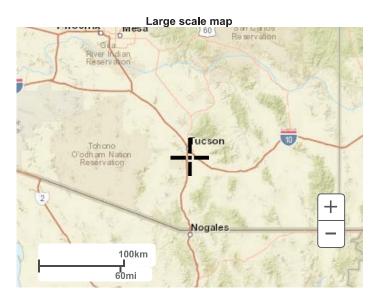
Back to Top

### Maps & aerials

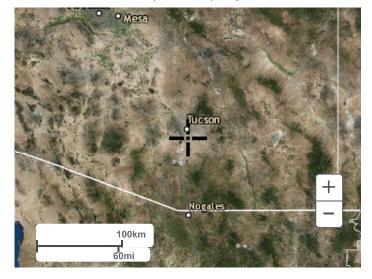
#### Small scale terrain







Large scale aerial



Back to Top

US Department of Commerce National Oceanic and Atmospheric Administration National Weather Service National Water Center 1325 East West Highway Silver Spring, MD 20910 Questions?: <u>HDSC.Questions@noaa.gov</u>

<u>Disclaimer</u>



NOAA Atlas 14, Volume 1, Version 5 Location name: Tucson, Arizona, USA\* Latitude: 32.1192°, Longitude: -110.9428° Elevation: 2592.28 ft\*\* \* source: ESRI Maps \*\* source: USGS



#### POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

PF\_tabular | PF\_graphical | Maps\_&\_aerials

### PF tabular

PDS-b	PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour) <sup>1</sup>									
Duration				Avera	ge recurren	ce interval (	years)			
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	<b>3.02</b> (2.71-3.41)	<b>3.89</b> (3.49-4.39)	<b>5.14</b> (4.58-5.77)	<b>6.08</b> (5.39-6.83)	<b>7.37</b> (6.44-8.24)	<b>8.34</b> (7.19-9.35)	<b>9.35</b> (7.93-10.5)	<b>10.3</b> (8.64-11.7)	<b>11.7</b> (9.50-13.3)	<b>12.7</b> (10.1-14.6)
10-min	<b>2.30</b>	<b>2.96</b>	<b>3.91</b>	<b>4.63</b>	<b>5.61</b>	<b>6.35</b>	<b>7.11</b>	<b>7.87</b>	<b>8.90</b>	<b>9.68</b>
	(2.06-2.60)	(2.66-3.35)	(3.48-4.39)	(4.09-5.20)	(4.90-6.27)	(5.47-7.12)	(6.04-8.01)	(6.57-8.91)	(7.23-10.2)	(7.70-11.1)
15-min	<b>1.90</b>	<b>2.44</b>	<b>3.23</b>	<b>3.82</b>	<b>4.64</b>	<b>5.24</b>	<b>5.88</b>	<b>6.51</b>	<b>7.35</b>	<b>8.00</b>
	(1.70-2.14)	(2.20-2.77)	(2.88-3.63)	(3.38-4.29)	(4.05-5.18)	(4.52-5.88)	(4.99-6.62)	(5.43-7.36)	(5.97-8.39)	(6.37-9.18)
30-min	<b>1.28</b>	<b>1.65</b>	<b>2.18</b>	<b>2.58</b>	<b>3.12</b>	<b>3.53</b>	<b>3.96</b>	<b>4.38</b>	<b>4.95</b>	<b>5.39</b>
	(1.15-1.44)	(1.48-1.86)	(1.94-2.45)	(2.28-2.89)	(2.73-3.49)	(3.04-3.96)	(3.36-4.46)	(3.66-4.96)	(4.02-5.65)	(4.29-6.18)
60-min	<b>0.791</b> (0.710-0.894)	<b>1.02</b> (0.914-1.15)	<b>1.35</b> (1.20-1.51)	<b>1.59</b> (1.41-1.79)	<b>1.93</b> (1.69-2.16)	<b>2.19</b> (1.88-2.45)	<b>2.45</b> (2.08-2.76)	<b>2.71</b> (2.26-3.07)	<b>3.06</b> (2.49-3.50)	<b>3.33</b> (2.65-3.83)
2-hr	<b>0.458</b>	<b>0.586</b>	<b>0.761</b>	<b>0.897</b>	<b>1.08</b>	<b>1.23</b>	<b>1.38</b>	<b>1.53</b>	<b>1.74</b>	<b>1.90</b>
	(0.414-0.515)	(0.528-0.658)	(0.682-0.852)	(0.796-1.00)	(0.954-1.21)	(1.07-1.37)	(1.18-1.54)	(1.29-1.71)	(1.42-1.96)	(1.52-2.16)
3-hr	<b>0.324</b>	<b>0.409</b>	<b>0.524</b>	<b>0.618</b>	<b>0.748</b>	<b>0.849</b>	<b>0.958</b>	<b>1.07</b>	<b>1.23</b>	<b>1.35</b>
	(0.292-0.363)	(0.369-0.460)	(0.472-0.589)	(0.551-0.692)	(0.659-0.834)	(0.738-0.946)	(0.816-1.07)	(0.893-1.20)	(0.992-1.39)	(1.07-1.55)
6-hr	<b>0.184</b>	<b>0.230</b>	<b>0.289</b>	<b>0.338</b>	<b>0.406</b>	<b>0.459</b>	<b>0.516</b>	<b>0.575</b>	<b>0.656</b>	<b>0.723</b>
	(0.166-0.206)	(0.207-0.258)	(0.259-0.324)	(0.301-0.378)	(0.356-0.453)	(0.398-0.512)	(0.439-0.576)	(0.481-0.644)	(0.533-0.742)	(0.575-0.826)
12-hr	<b>0.103</b>	<b>0.129</b>	<b>0.160</b>	<b>0.185</b>	<b>0.221</b>	<b>0.248</b>	<b>0.276</b>	<b>0.306</b>	<b>0.345</b>	<b>0.377</b>
	(0.093-0.115)	(0.117-0.144)	(0.144-0.178)	(0.166-0.206)	(0.195-0.245)	(0.217-0.276)	(0.238-0.309)	(0.258-0.344)	(0.285-0.392)	(0.305-0.432)
24-hr	<b>0.058</b>	<b>0.073</b>	<b>0.091</b>	<b>0.105</b>	<b>0.125</b>	<b>0.141</b>	<b>0.157</b>	<b>0.174</b>	<b>0.197</b>	<b>0.215</b>
	(0.053-0.064)	(0.067-0.080)	(0.083-0.099)	(0.096-0.115)	(0.113-0.138)	(0.127-0.155)	(0.140-0.174)	(0.153-0.194)	(0.171-0.222)	(0.184-0.244)
2-day	<b>0.032</b>	<b>0.040</b>	<b>0.049</b>	<b>0.057</b>	<b>0.068</b>	<b>0.077</b>	<b>0.086</b>	<b>0.095</b>	<b>0.108</b>	<b>0.118</b>
	(0.029-0.035)	(0.036-0.043)	(0.045-0.054)	(0.053-0.063)	(0.062-0.075)	(0.069-0.085)	(0.077-0.095)	(0.084-0.106)	(0.094-0.122)	(0.101-0.135)
3-day	<b>0.022</b>	<b>0.028</b>	<b>0.035</b>	<b>0.041</b>	<b>0.049</b>	<b>0.056</b>	<b>0.063</b>	<b>0.070</b>	<b>0.080</b>	<b>0.089</b>
	(0.021-0.025)	(0.026-0.031)	(0.032-0.038)	(0.037-0.045)	(0.045-0.054)	(0.050-0.061)	(0.056-0.070)	(0.061-0.078)	(0.069-0.091)	(0.075-0.101)
4-day	<b>0.018</b>	<b>0.022</b>	<b>0.028</b>	<b>0.033</b>	<b>0.040</b>	<b>0.045</b>	<b>0.051</b>	<b>0.058</b>	<b>0.067</b>	<b>0.074</b>
	(0.016-0.020)	(0.020-0.024)	(0.026-0.031)	(0.030-0.036)	(0.036-0.043)	(0.040-0.050)	(0.045-0.057)	(0.050-0.064)	(0.057-0.075)	(0.062-0.084)
7-day	<b>0.012</b>	<b>0.015</b>	<b>0.018</b>	<b>0.022</b>	<b>0.026</b>	<b>0.030</b>	<b>0.034</b>	<b>0.039</b>	<b>0.045</b>	<b>0.050</b>
	(0.011-0.013)	(0.013-0.016)	(0.017-0.020)	(0.020-0.024)	(0.024-0.029)	(0.027-0.033)	(0.030-0.038)	(0.034-0.043)	(0.038-0.051)	(0.042-0.058)
10-day	<b>0.009</b>	<b>0.011</b>	<b>0.014</b>	<b>0.017</b>	<b>0.020</b>	<b>0.023</b>	<b>0.026</b>	<b>0.029</b>	<b>0.034</b>	<b>0.038</b>
	(0.008-0.010)	(0.010-0.013)	(0.013-0.016)	(0.015-0.018)	(0.018-0.022)	(0.021-0.025)	(0.023-0.029)	(0.025-0.033)	(0.029-0.038)	(0.031-0.043)
20-day	<b>0.006</b>	<b>0.008</b>	<b>0.009</b>	<b>0.011</b>	<b>0.013</b>	<b>0.015</b>	<b>0.017</b>	<b>0.018</b>	<b>0.021</b>	<b>0.023</b>
	(0.006-0.007)	(0.007-0.008)	(0.009-0.010)	(0.010-0.012)	(0.012-0.014)	(0.013-0.016)	(0.015-0.018)	(0.016-0.020)	(0.018-0.024)	(0.019-0.026)
30-day	<b>0.005</b>	<b>0.006</b>	<b>0.007</b>	<b>0.009</b>	<b>0.010</b>	<b>0.011</b>	<b>0.012</b>	<b>0.014</b>	<b>0.015</b>	<b>0.016</b>
	(0.004-0.005)	(0.006-0.007)	(0.007-0.008)	(0.008-0.009)	(0.009-0.011)	(0.010-0.012)	(0.011-0.014)	(0.012-0.015)	(0.013-0.017)	(0.014-0.019)
45-day	<b>0.004</b>	<b>0.005</b>	<b>0.006</b>	<b>0.007</b>	<b>0.008</b>	<b>0.009</b>	<b>0.009</b>	<b>0.010</b>	<b>0.011</b>	<b>0.012</b>
	(0.004-0.004)	(0.005-0.005)	(0.006-0.006)	(0.006-0.007)	(0.007-0.009)	(0.008-0.009)	(0.009-0.010)	(0.009-0.011)	(0.010-0.012)	(0.010-0.013)
60-day	<b>0.003</b>	<b>0.004</b>	<b>0.005</b>	<b>0.006</b>	<b>0.007</b>	<b>0.007</b>	<b>0.008</b>	<b>0.009</b>	<b>0.009</b>	<b>0.010</b>
	(0.003-0.004)	(0.004-0.005)	(0.005-0.006)	(0.005-0.006)	(0.006-0.007)	(0.007-0.008)	(0.007-0.009)	(0.008-0.009)	(0.008-0.010)	(0.009-0.011)

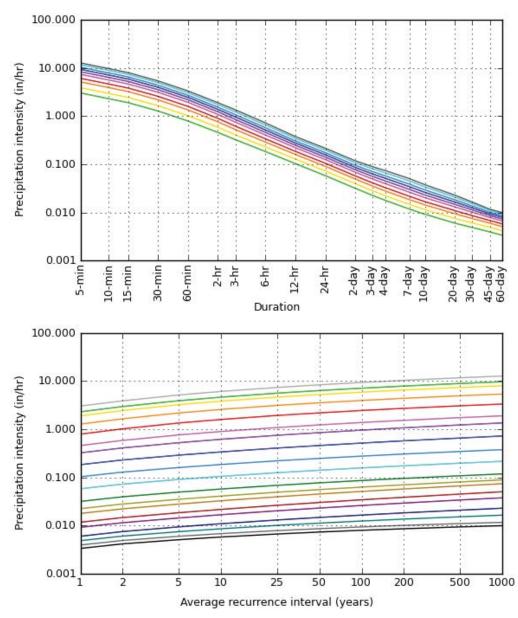
<sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

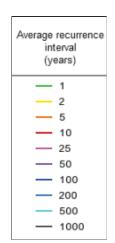
Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

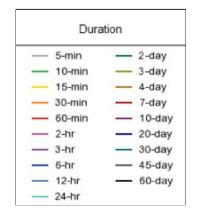
Back to Top

## **PF graphical**

PDS-based intensity-duration-frequency (IDF) curves Latitude: 32.1192°, Longitude: -110.9428°







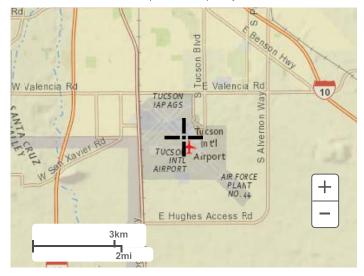
NOAA Atlas 14, Volume 1, Version 5

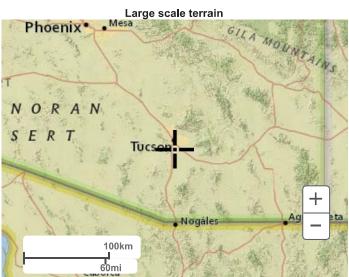
Created (GMT): Thu Jul 20 16:19:45 2017

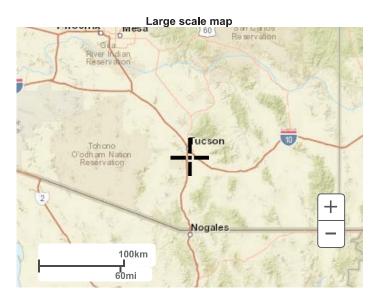
Back to Top

### Maps & aerials

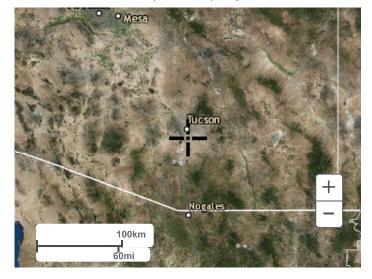
#### Small scale terrain







Large scale aerial



Back to Top

US Department of Commerce National Oceanic and Atmospheric Administration National Weather Service National Water Center 1325 East West Highway Silver Spring, MD 20910 Questions?: <u>HDSC.Questions@noaa.gov</u>

<u>Disclaimer</u>

Tucson International Airport

Conceptual Drainage Design



# Appendix A.2 – Calculations

Project: Tucson International Airport Subject: Summary Author: CGC Location: Tucson, Arizona Date: 11/21/2017

Reference:	11/21/2017 Standards Manual for Dr	ainage Desigr	and Floodplain Mana	gement In Tucson, Ariz	ona; July 1998 (eff).		
*During a 4 mar 10	*Area Total	Area	Contributing Flow	Contributing Flow	Pervious Area/	*Area	Area
*Drainage Area ID	(sf)	(acres)	5 - Year (cfs)	100 - Year (cfs)	Storage ID	(sf)	(acres)
DA-01	1,234,628	28.3	18.9	53.9	SA-01	699,488	16.1
DA-02	312,909	7.2	8.7	24.7	SA-02	140,807	3.2
DA-03	430,629	9.9	10.8	30.8	SA-03	240,737	5.5
DA-04	289,581	6.6	7.4	21.2	SA-04	126,332	2.9
DA-05	145,683	3.3	4.2	12.1	SA-05	63,978	1.5
DA-06	352,978	8.1	9.0	25.7	SA-06	146,755	3.4
DA-07	355,831	8.2	9.1	26.1	SA-07	142,623	3.3
DA-08	309,682	7.1	8.5	24.2	SA-08	140,681	3.2
DA-09	300,598	6.9	8.5	24.2	SA-09	114,913	2.6
DA-10	302,198	6.9	8.4	23.9	SA-10	113,362	2.6
DA-11	717,615	16.5	13.6	38.8	SA-11	411,303	9.4
DA-12	1,780,985	40.9	31.9	91.1	SA-12	1,210,237	27.8
DA-13	801,867	18.4	15.4	43.9	SA-13	408,631	9.4
DA-14	802,726	18.4	15.7	44.8	SA-14	409,985	9.4
DA-15	611,475	14.0	12.5	35.7	SA-15	343,285	7.9
DA-16	391,502	9.0	8.4	24.1	SA-16	160,736	3.7
DA-17	1,008,430	23.2	17.2	49.2	SA-17	527,539	12.1
DA-18	739,420	17.0	12.3	35.2	SA-18	385,315	8.8
DA-19	644,703	14.8	12.8	36.5	SA-19	355,405	8.2
DA-20	672,693	15.4	14.7	41.9	SA-20	332,756	7.6
DA-21	542,386	12.5	11.5	33.0	SA-21	252,134	5.8
DA-22	777,165	17.8	14.4	41.1	SA-22	423,544	9.7
DA-23	428,416	9.8	8.3	23.7	SA-23	193,213	4.4
DA-24	610,310	14.0	10.9	31.1	SA-24	314,103	7.2
DA-25	534,241	12.3	10.6	30.4	SA-25	243,950	5.6
DA-26	365,559	8.4	9.3	26.5	SA-26	82,288	1.9
DA-27	302,592	6.9	7.6	21.7	SA-27	82,770	1.9
DA-28	430,460	9.9	12.0	34.2	SA-28	125,929	2.9
DA-29	948,833	21.8	18.4	52.5	SA-29	550,958	12.6
DA-30	276,453	6.3	6.9	19.8	SA-30	80,355	1.8
DA-31	1,124,933	25.8	21.5	61.4	SA-31	655,267	15.0
DA-32	1,119,211	25.7	21.5	61.6	SA-32	649,816	14.9
DA-33	1,036,665	23.8	21.5	61.3	SA-33	632,269	14.5
DA-34	1,318,983	30.3	24.0	68.5	SA-34	802,107	18.4
DA-35	506,488	11.6	12.9	36.7	SA-35	232,215	5.3
DA-36	256,513	5.9	7.8	22.2	SA-36	102,295	2.3
DA-37	313,944	7.2	7.5	21.5	SA-37	184,219	4.2
(PROP) AF-DA-1	1,820,116	41.8	27.4	78.4	N/A	N/A	N/A
(PROP) AF-DA-2	7,007,424	160.9	95.1	271.6	N/A	N/A	N/A
(PROP) AF-DA-3	3,368,198	77.3	40.2	115.0	N/A	N/A	N/A
(PROP) AF-DA-4	25,650,188	588.8	173.3	495.0	N/A	N/A	N/A
(PROP) AF-DA-5	27,142,971	623.1	135.6	387.3	N/A	N/A	N/A
(PROP) AF-DA-6	2,823,329	64.8	27.4	78.4	N/A	N/A	N/A
(EX) AF-DA-1	1,820,116	41.8	27.4	78.4	N/A	N/A	N/A
(EX) AF-DA-2	7,007,424	160.9	95.1	271.6	N/A	N/A	N/A
(EX) AF-DA-3	3,368,198	77.3	40.2	115.0	N/A	N/A	N/A
(EX) AF-DA-4	26,952,645	618.7	165.9	474.0	N/A	N/A	N/A
(EX) AF-DA-5	25,845,028	593.3	119.5	341.3	N/A	N/A	N/A
(EX) AF-DA-6	2,823,329	64.8	40.22582117	114.9309176	N/A	N/A	N/A

Notes: 1)

\* = User Input Required

## Project: Tucson International Airport Subject: Summary Author: CGC Location: Tucson, Arizona Date: 11/21/2017

Reference:

Standards Manual for Drainage Design and Floodplain Management In Tucson, Arizona; July 1998 (eff).

DA Group - Discharge Pt.	Drainage Area ID	Area (acres)	Outlet Slope (ft/ft)	Discharge - 5 yr (cfs)	Pipe Size Required (in)
	8	7.1	0.0073	7.8	18
	8,5	10.5	0.0062	9.7	24
1	8,5,3	20.3	0.0058	15.3	24
	8,5,3,2	27.5	0.0060	18.7	24
	8,5,3,2,1*	55.9	0.0037	21.8	30
	6	8.1	0.0047	8.1	18
2	7	8.2	0.0070	8.8	18
	6,7,4*	22.9	0.0058	18.7	30
	11	16.5	0.0071	13.5	24
	11,10	23.4	0.0066	17.1	24
	11,10,9	30.3	0.0061	19.7	24
3	15	14.0	0.0061	11.8	24
	15,14	32.5	0.0064	20.7	24
	15,14,13	50.9	0.0068	31.1	30
	15,14,13,11,10,9,12*	122.1	0.0068	62.9	42
	27	6.9	0.0082	6.2	18
	27,24	21.0	0.0058	12.1	24
	26	8.4	0.0117	8.2	18
	26,25	20.7	0.0085	14.6	24
	27,26,25,24,23	51.4	0.0061	27.1	30
л	27,26,25,24,23,21	63.9	0.0064	29.1	30
4	27,26,25,24,23,21,20	79.3	0.0065	34.3	30
	22	17.8	0.0068	14.2	24
	19	14.8	0.0058	12.0	24
	22,19,18	49.6	0.0061	27.2	30
	16	9.0	0.0049	0.2	24
	27,26,25,24,23,22,21,20,19,18,16,17*	161.1	0.0064	58.1	42

Project: Tucson International Airport Subject: Onsite Calculations Author: CGC Location: Tucson, Arizona Date: 11/21/2017

Reference: Standards Manual for Drainage Design and Floodplain Management In Tucson, Arizona; July 1998 (eff).

															Table 4.1	1	Table 4.2	1	Table 4.3	Table 4.4				Table 4.5
Drainage Area ID	Area (acres)	*L <sub>c</sub> (ft)	*∆H (ft)	L <sub>ca</sub> (feet)	∗∆L₁ (feet)	$^{*\Delta H_1}$ (feet)	*ΔL <sub>2</sub> (feet)	$*\Delta H_2$ (feet)	*ΔL <sub>3</sub> (feet)	*∆H <sub>3</sub> (feet)	∗∆L₄ (feet)	*∆H₄ (feet)	G	S <sub>c</sub> (ft/ft)	*P <sub>1.100</sub>	*Watershed Type	*n <sub>b100</sub>	*Soil Type	C <sub>w100</sub>	*F <sub>ac</sub>	T <sub>c100</sub> (minutes)	i <sub>100</sub> (in/hr)	Q <sub>p100</sub> (cfs)	Q <sub>p5</sub> (cfs)
DA-01	28.3	2064	8	1032	516	2	516	2	516	2	516	2	33152.75	0.0039	1.5	Comm./Ind.	0.048	D	0.83	1	32.5	2.3	53.9	18.9
DA-02 DA-03	7.2 9.9	606 847	4 6	303 424	151.5 211.75	1 1.5	151.5 211.75	1 1.5	151.5 211.75	1 1.5	151.5 211.75	1 1.5	7458.971 10063.51	0.0066	1.5 1.5	Comm./Ind. Comm./Ind.	0.048 0.048	D D	0.86 0.83	1 1	9.8 12.1	4.0 3.7	24.7 30.8	8.7 10.8
DA-03 DA-04	9.9 6.6	737	4	369	184.25	1.5	184.25	1.5	184.25	1.5	184.25	1.5	10003.91	0.0071	1.5	Comm./Ind.	0.048	D	0.85	1	12.1	3.7	21.2	7.4
DA-05	3.3	403	2	202	100.75	0.5	100.75	0.5	100.75	0.5	100.75	0.5	5720.613	0.0050	1.5	Comm./Ind.	0.048	D	0.86	1	8.5	4.2	12.1	4.2
DA-06	8.1	829	5	415	207.25	1.25	207.25	1.25	207.25	1.25	207.25	1.25	10674.48	0.0060	1.5	Comm./Ind.	0.048	D	0.86	1	12.6	3.7	25.7	9.0
DA-07	8.2	822	5	411	205.5	1.25	205.5	1.25	205.5	1.25	205.5	1.25	10539.57	0.0061	1.5	Comm./Ind.	0.048	D	0.87	1	12.5	3.7	26.1	9.1
DA-08	7.1	818	8	409	204.5	2	204.5	2	204.5	2	204.5	2	8271.513	0.0098	1.5	Comm./Ind.	0.048	D	0.85	1	10.1	4.0	24.2	8.5
DA-09	6.9	716	6	358	179	1.5	179	1.5	179	1.5	179	1.5	7821.569	0.0084	1.5	Comm./Ind.	0.048	D	0.87	1	9.8	4.0	24.2	8.5
DA-10 DA-11	6.9 16.5	699 1946	5 14	350 973	174.75 486.5	1.25 3.5	174.75 486.5	1.25 3.5	174.75 486.5	1.25 3.5	174.75 486.5	1.25 3.5	8264.77 22943	0.0072	1.5 1.5	Comm./Ind. Comm./Ind.	0.048	D D	0.87 0.83	1	10.3 22.3	4.0 2.8	23.9 38.8	8.4 13.6
DA-11 DA-12	40.9	2004	14	1002	480.5 501	3.5	501	3.5	501	3.5	480.5 501	3.5	23976.32	0.0072	1.5	Comm./Ind.	0.048	D	0.85	1	22.5	2.8	91.1	31.9
DA-12 DA-13	18.4	1912	13	956	478	3.25	478	3.25	478	3.25	478	3.25	23187.83	0.0068	1.5	Comm./Ind.	0.048	D	0.84	1	22.4	2.8	43.9	15.4
DA-14	18.4	1911	14	956	477.75	3.5	477.75	3.5	477.75	3.5	477.75	3.5	22326.83	0.0073	1.5	Comm./Ind.	0.048	D	0.84	1	21.6	2.9	44.8	15.7
DA-15	14.0	1636	12	818	409	3	409	3	409	3	409	3	19102.24	0.0073	1.5	Comm./Ind.	0.048	D	0.83	1	19.3	3.1	35.7	12.5
DA-16	9.0	1225	6	613	306.25	1.5	306.25	1.5	306.25	1.5	306.25	1.5	17503.65	0.0049	1.5	Comm./Ind.	0.048	D	0.86	1	18.6	3.1	24.1	8.4
DA-17	23.2	2416	16	1208	604	4	604	4	604	4	604	4	29688.31	0.0066	1.5	Comm./Ind.	0.048	D	0.84	1	27.4	2.5	49.2	17.2
DA-18	17.0	2064	10	1032	516	2.5	516	2.5 3	516	2.5 3	516	2.5	29652.72	0.0048	1.5	Comm./Ind.	0.048	D	0.84	1	28.6	2.5	35.2	12.3
DA-19 DA-20	14.8 15.4	1726 1619	12 14	863 810	431.5 404.75	3 3.5	431.5 404.75	3.5	431.5 404.75	3.5	431.5 404.75	3 3.5	20700.01 17410.3	0.0070 0.0086	1.5 1.5	Comm./Ind. Comm./Ind.	0.048 0.048	D D	0.83 0.85	1	20.6 17.4	3.0 3.2	36.5 41.9	12.8 14.7
DA-20 DA-21	12.5	1369	8	685	342.25	2	342.25	2	342.25	2	342.25	2	17908.54	0.0058	1.5	Comm./Ind.	0.048	D	0.85	1	17.4	3.1	33.0	14.7
DA-22	17.8	2032	14	1016	508	3.5	508	3.5	508	3.5	508	3.5	24480.57	0.0069	1.5	Comm./Ind.	0.048	D	0.84	1	23.5	2.8	41.1	14.4
DA-23	9.8	1315	5	658	328.75	1.25	328.75	1.25	328.75	1.25	328.75	1.25	21325.72	0.0038	1.5	Comm./Ind.	0.048	D	0.86	1	22.5	2.8	23.7	8.3
DA-24	14.0	1543	6	772	385.75	1.5	385.75	1.5	385.75	1.5	385.75	1.5	24744.19	0.0039	1.5	Comm./Ind.	0.048	D	0.84	1	25.4	2.6	31.1	10.9
DA-25	12.3	1358	6	679	339.5	1.5	339.5	1.5	339.5	1.5	339.5	1.5	20430.27	0.0044	1.5	Comm./Ind.	0.048	D	0.85	1	21.4	2.9	30.4	10.6
DA-26	8.4	1629	20	815	407.25	5	407.25	5	407.25	5	407.25	5	14701.67	0.0123	1.5	Comm./Ind.	0.048	D	0.90	1	14.3	3.5	26.5	9.3
DA-27 DA-28	6.9 9.9	1626 786	20 6	813 393	406.5 196.5	5 1.5	406.5 196.5	5 1.5	406.5 196.5	5 1.5	406.5 196.5	5 1.5	14661.07 8996.181	0.0123	1.5 1.5	Comm./Ind. Comm./Ind.	0.048 0.048	D D	0.89 0.89	1 1	14.3 10.8	3.5 3.9	21.7 34.2	7.6 12.0
DA-28 DA-29	21.8	1499	8	750	374.75	2	374.75	2	374.75	2	374.75	2	20519.06	0.0078	1.5	Comm./Ind.	0.048	D	0.89	1	21.3	2.9	52.5	12.0
DA-30	6.3	996	6	498	249	1.5	249	1.5	249	1.5	249	1.5	12832.56	0.0060	1.5	Comm./Ind.	0.048	D	0.89	1	14.2	3.5	19.8	6.9
DA-31	25.8	1798	12	899	449.5	3	449.5	3	449.5	3	449.5	3	22008.68	0.0067	1.5	Comm./Ind.	0.048	D	0.83	1	21.8	2.9	61.4	21.5
DA-32	25.7	1777	12	889	444.25	3	444.25	3	444.25	3	444.25	3	21624.22	0.0068	1.5	Comm./Ind.	0.048	D	0.83	1	21.5	2.9	61.6	21.5
DA-33	23.8	1613	13	807	403.25	3.25	403.25	3.25	403.25	3.25	403.25	3.25	17967.18	0.0081	1.5	Comm./Ind.	0.048	D	0.82	1	18.3	3.1	61.3	21.5
DA-34	30.3	2083	15	1042	520.75	3.75	520.75	3.75	520.75	3.75	520.75	3.75	24546.43	0.0072	1.5	Comm./Ind.	0.048	D	0.82	1	23.6	2.8	68.5	24.0
DA-35 DA-36	11.6 5.9	745 477	4	373 239	186.25 119.25	1	186.25 119.25	1 1	186.25 119.25	1 1	186.25 119.25	1	10167.27 5208.919	0.0054 0.0084	1.5 1.5	Comm./Ind. Comm./Ind.	0.048 0.048	D D	0.85 0.87	1 1	12.4 7.6	3.7 4.3	36.7 22.2	12.9 7.8
DA-36 DA-37	7.2	695	4	348	173.75	0.75	173.75	0.75	173.75	0.75	173.75	0.75	10578.32	0.0084	1.5	Comm./Ind.	0.048	D	0.87	1	13.3	4.5 3.6	22.2	7.8
(PROP) AF-DA-1	41.8	2236	16	1118	559	4	559	4	559	4	559	4	26433.08	0.0072	1.5	Comm./Ind.	0.048	D	0.73	1	26.7	2.6	78.4	27.4
(PROP) AF-DA-2	160.9	4027	28	2014	1006.75	7	1006.75	7	1006.75	7	1006.75	7	48294.03	0.0070	1.5	Comm./Ind.	0.048	D	0.85	1	40.4	2.0	271.6	95.1
(PROP) AF-DA-3	77.3	3962	24	1981	990.5	6	990.5	6	990.5	6	990.5	6	50905.66	0.0061	1.5	Comm./Ind.	0.048	D	0.80	1	44.6	1.9	115.0	40.2
(PROP) AF-DA-4	588.8	9456	52	4728	2364	13	2364	13	2364	13	2364	13	127514.5	0.0055	1.5	Comm./Ind.	0.048	D	0.83	1	98.5	1.0	495.0	173.3
(PROP) AF-DA-5	623.1	15701	110	7850.5	3925.25	27.5	3925.25	27.5	3925.25	27.5	3925.25	27.5	187583.5	0.0070	1.5	Comm./Ind.	0.048	D	0.81	1	136.4	0.8	387.3	135.6
(PROP) AF-DA-6	64.8	3528	22	1764	882	5.5	882	5.5	882	5.5	882	5.5	44676.74	0.0062	1.5	Comm./Ind.	0.048	D	0.86	1	38.2	2.1	114.9	40.2
(EX) AF-DA-1	41.8	2236	16	1118	559 1006 75	4 7	559 1006 75	4 7	559 1006 75	4	559	4 7	26433.08 48294.03	0.0072	1.5	Comm./Ind.	0.048	D	0.73	1	26.7 40.4	2.6	78.4	27.4
(EX) AF-DA-2 (EX) AF-DA-3	160.9 77.3	4027 3962	28 24	2014 1981	1006.75 990.5	6	1006.75 990.5	6	1006.75 990.5	7	1006.75 990.5	6	48294.03 50905.66	0.0070	1.5 1.5	Comm./Ind. Comm./Ind.	0.048 0.048	D D	0.85 0.80	1 1	40.4 44.6	2.0 1.9	271.6 115.0	95.1 40.2
(EX) AF-DA-3 (EX) AF-DA-4	618.7	9456	52	4728	2364	13	2364	13	2364	13	2364	13	127514.5	0.0055	1.5	Comm./Ind.	0.048	D	0.80	1	102.2	1.9	474.0	165.9
(EX) AF-DA-5	593.3	15701	110	7850.5	3925.25	27.5	3925.25	27.5	3925.25	27.5	3925.25	27.5	187583.5	0.0070	1.5	Comm./Ind.	0.048	D	0.77	1	140.6	0.7	341.3	119.5
(EX) AF-DA-6	64.8	3528	22	1764	882	5.5	882	5.5	882	5.5	882	5.5	44676.74	0.0062	1.5	Comm./Ind.	0.048	D	0.86	1	38.2	2.1	114.9	40.2

Notes:

1) \* = User Input Required

Project: Tucson International Airport Subject: Weighted C-Values Author: CGC Location: Tucson, Arizona Date: 11/21/2017

Reference: Standards Manual for Drainage Design and Floodplain Management In Tucson, Arizona; July 1998 (eff)

Drainage	*Surface 1 Coeff.	*Surface 1	*Surface 2 Coeff.	*Surface 2	Weighted C
Area ID	Asphalt/Concrete	Area (sf)	Natural Ground	Area (sf)	Value
DA-01	0.95	535,140	0.74	699,488	0.83
DA-02	0.95	172,102	0.74	140,807	0.86
DA-03	0.95	189,892	0.74	240,737	0.83
DA-04	0.95	163,249	0.74	126,332	0.86
DA-05	0.95	81,705	0.74	63,978	0.86
DA-06	0.95	206,223	0.74	146,755	0.86
DA-07	0.95	213,208	0.74	142,623	0.87
DA-08	0.95	169,001	0.74	140,681	0.85
DA-09	0.95	185,685	0.74	114,913	0.87
DA-10	0.95	188,836	0.74	113,362	0.87
DA-11	0.95	306,312	0.74	411,303	0.83
DA-12	0.95	570,748	0.74	1,210,237	0.81
DA-13	0.95	393,236	0.74	408,631	0.84
DA-14	0.95	392,741	0.74	409,985	0.84
DA-15	0.95	268,190	0.74	343,285	0.83
DA-16	0.95	230,766	0.74	160,736	0.86
DA-17	0.95	480,891	0.74	527,539	0.84
DA-18	0.95	354,105	0.74	385,315	0.84
DA-19	0.95	289,298	0.74	355,405	0.83
DA-20	0.95	339,937	0.74	332,756	0.85
DA-21	0.95	290,252	0.74	252,134	0.85
DA-22	0.95	353,621	0.74	423,544	0.84
DA-23	0.95	235,203	0.74	193,213	0.86
DA-24	0.95	296,207	0.74	314,103	0.84
DA-25	0.95	290,291	0.74	243,950	0.85
DA-26	0.95	283,271	0.74	82,288	0.90
DA-27	0.95	219,822	0.74	82,770	0.89
DA-28	0.95	304,531	0.74	125,929	0.89
DA-29	0.95	397,875	0.74	550,958	0.83
DA-30	0.95	196,098	0.74	80,355	0.89
DA-31	0.95	469,666	0.74	655,267	0.83
DA-32	0.95	469,395	0.74	649,816	0.83
DA-33	0.95	404,396	0.74	632,269	0.82
DA-34	0.95	516,876	0.74	802,107	0.82
DA-35	0.95	274,273	0.74	232,215	0.85
DA-36	0.95	154,218	0.74	102,295	0.87
DA-37	0.95	129,725	0.74	184,219	0.83

Notes: 1)

\* = User Input Required

2) Airfield C-Values were calculated in another spreadsheet, also found in the appendix.

Project: Tucson International Airport Subject: Ratio Factors from Table 4.5 Author: CGC .ocation: Tucson, Arizona Date: 11/21/2017

Reference: Standards Manual for Drainage Design and Floodplain Management In Tucson, Arizona; July 1998 (eff).

Drainage	
Area ID	*Factor from Table 4.5
DA-01	0.35
DA-02	0.35
DA-02 DA-03	0.35
DA-03	0.35
DA-04 DA-05	0.35
DA-05 DA-06	0.35
DA-07	0.35
DA-08	0.35
DA-09	0.35
DA-10	0.35
DA-11	0.35
DA-12	0.35
DA-13	0.35
DA-14	0.35
DA-15	0.35
DA-16	0.35
DA-17	0.35
DA-18	0.35
DA-19	0.35
DA-20	0.35
DA-21	0.35
DA-22	0.35
DA-23	0.35
DA-24	0.35
DA-25	0.35
DA-26	0.35
DA-27	0.35
DA-28	0.35
DA-29	0.35
DA-30	0.35
DA-31	0.35
DA-32	0.35
DA-33	0.35
DA-34	0.35
DA-35	0.35
DA-36	0.35
DA-37	0.35
(PROP) AF-DA-1	0.35
(PROP) AF-DA-2	0.35
(PROP) AF-DA-3	0.35
(PROP) AF-DA-4	0.35
(PROP) AF-DA-5	0.35
(PROP) AF-DA-6	0.35
(EX) AF-DA-1	0.35
(EX) AF-DA-2	0.35
(EX) AF-DA-3	0.35
(EX) AF-DA-4	0.35
(EX) AF-DA-5	0.35
(EX) AF-DA-6	0.35

Project: Tucson International Airport Subject: Pipe Sizing Author: CGC Location: Tucson, Arizona Date: 11/21/2017 Standards Manual for Drainage Design and Floodplain Management In Tucson, Arizona; July 1998 (eff). Reference:

Project: Tucson International Airport Subject: Ratio Factors from Table 4.5 Author: CGC Location: Tucson, Arizona Date: 11/12/12017 Reference: Standards Manual for Drainage Design and Floodplain Management In Tucson, Arizona; July 1998 (eff).

															Table 4.1		Table 4.2		Table 4.3	Table 4.4				Table 4.5		
DA Group - Discharge Pt.	Drainage Area ID	Area (acres)	*L <sub>c</sub> (ft)	*∆H (ft)	L <sub>ca</sub> (feet)	*ΔL <sub>1</sub> (feet)	$^{\circ \Delta H_1}$ (feet)	*ΔL <sub>2</sub> (feet)	$\Delta H_2$ (feet)	*ΔL <sub>3</sub> (feet)	*ΔH <sub>3</sub> (feet)	*∆L <sub>4</sub> (feet)	*∆H₄ (feet)	G	S <sub>c</sub> (ft/ft)	*P <sub>1.100</sub>	*Watershed Type	*n <sub>b100</sub>	*Soil Type	C <sub>w100</sub>	*F <sub>ac</sub>	T <sub>c100</sub> (minutes)	i <sub>100</sub> (in/hr)	Q <sub>p100</sub> (cfs)	Q <sub>p5</sub> (cfs)	Pipe Size Needed (Inches)
	8	7.1	821	6	410.5	205.25	1.5	205.25	1.5	205.25	1.5	205.25	1.5	9603.7	0.007	1.5	Comm./Ind.	0.048	D	0.83	1	11.7	3.8	22.3	7.8	18
	8,5	10.5	1286	8	643	321.5	2	321.5	2	321.5	2	321.5	2	16304.8	0.006	1.5	Comm./Ind.	0.048	D	0.83	1	17.5	3.2	27.8	9.7	24
1	8,5,3	20.3	2073	12	1036.5	518.25	3	518.25	3	518.25	3	518.25	3	27246.3	0.006	1.5	Comm./Ind.	0.048	D	0.83	1	26.3	2.6	43.8	15.3	24
	8,5,3,2	27.5	2654	16	1327	663.5	4	663.5	4	663.5	4	663.5	4	34181.5	0.006	1.5	Comm./Ind.	0.048	D	0.83	1	31.4	2.3	53.4	18.7	24
	8,5,3,2,1*	55.9	4880	18	2440	1220	4.5	1220	4.5	1220	4.5	1220	4.5	80351.4	0.004	1.5	Comm./Ind.	0.048	D	0.83	1	69.4	1.3	62.2	21.8	30
	6	8.1	856	4	428	214	1	214	1	214	1	214	1	12522.2	0.005	1.5	Comm./Ind.	0.048	D	0.83	1	14.9	3.4	23.1	8.1	18
2	7	8.2	863	6	431.5	215.75	1.5	215.75	1.5	215.75	1.5	215.75	1.5	10350.0	0.007	1.5	Comm./Ind.	0.048	D	0.83	1	12.4	3.7	25.1	8.8	18
	6,7,4*	22.9	1728	10	864	432	2.5	432	2.5	432	2.5	432	2.5	22715.1	0.006	1.5	Comm./Ind.	0.048	D	0.83	1	22.7	2.8	53.4	18.7	30
	11	16.5	1959	14	979.5	489.75	3.5	489.75	3.5	489.75	3.5	489.75	3.5	23173.3	0.007	1.5	Comm./Ind.	0.048	D	0.83	1	22.5	2.8	38.6	13.5	24
	11,10	23.4	2416	16	1208	604	4	604	4	604	4	604	4	29688.3	0.007	1.5	Comm./Ind.	0.048	D	0.83	1	27.6	2.5	49.0	17.1	24
	11,10,9	30.3	2933	18	1466.5	733.25	4.5	733.25	4.5	733.25	4.5	733.25	4.5	37439.7	0.006	1.5	Comm./Ind.	0.048	D	0.83	1	33.7	2.2	56.2	19.7	24
3	15	14.0	1646	10	823	411.5	2.5	411.5	2.5	411.5	2.5	411.5	2.5	21117.6	0.006	1.5	Comm./Ind.	0.048	D	0.83	1	21.3	2.9	33.8	11.8	24
	15,14	32.5	3124	20	1562	781	5	781	5	781	5	781	5	39043.8	0.006	1.5	Comm./Ind.	0.048	D	0.83	1	34.7	2.2	59.1	20.7	24
	15,14,13	50.9	3525	24	1762.5	881.25	6	881.25	6	881.25	6	881.25	6	42720.2	0.007	1.5	Comm./Ind.	0.048	D	0.83	1	37.1	2.1	88.8	31.1	30
	15,14,13,11,10,9,12*	122.1	4731	32	2365.5	1182.75	8	1182.75	8	1182.75	8	1182.75	8	57524.7	0.007	1.5	Comm./Ind.	0.048	D	0.83	1	47.7	1.8	179.7	62.9	42
	27	6.9	1713	14	856.5	428.25	3.5	428.25	3.5	428.25	3.5	428.25	3.5	18948.4	0.008	1.5	Comm./Ind.	0.048	D	0.83	1	18.9	3.1	17.8	6.2	18
	27,24	21.0	3472	20	1736	868	5	868	5	868	5	868	5	45746.2	0.006	1.5	Comm./Ind.	0.048	D	0.83	1	40.2	2.0	34.7	12.1	24
	26	8.4	1709	20	854.5	427.25	5	427.25	5	427.25	5	427.25	5	15797.9	0.012	1.5	Comm./Ind.	0.048	D	0.83	1	15.8	3.4	23.4	8.2	18
	26,25	20.7	3054	26	1527	763.5	6.5	763.5	6.5	763.5	6.5	763.5	6.5	33099.2	0.009	1.5	Comm./Ind.	0.048	D	0.83	1	29.2	2.4	41.8	14.6	24
	27,26,25,24,23	51.4	4252	26	2126	1063	6.5	1063	6.5	1063	6.5	1063	6.5	54375.5	0.006	1.5	Comm./Ind.	0.048	D	0.83	1	46.1	1.8	77.5	27.1	30
4	27,26,25,24,23,21	63.9	5596	36	2798	1399	9	1399	9	1399	9	1399	9	69769.5	0.006	1.5	Comm./Ind.	0.048	D	0.83	1	56.7	1.6	83.0	29.1	30
	27,26,25,24,23,21,20	79.3	6117	40	3058.5	1529.25	10	1529.25	10	1529.25	10	1529.25	10	75644.6	0.007	1.5	Comm./Ind.	0.048	D	0.83	1	60.6	1.5	98.0	34.3	30
	22	17.8	2045	14	1022.5	511.25	3.5	511.25	3.5	511.25	3.5	511.25	3.5	24715.9	0.007	1.5	Comm./Ind.	0.048	D	0.83	1	23.8	2.7	40.6	14.2	24
	19	14.8	1737	10	868.5	434.25	2.5	434.25	2.5	434.25	2.5	434.25	2.5	22892.8	0.006	1.5	Comm./Ind.	0.048	D	0.83	1	22.9	2.8	34.4	12.0	24
	22,19,18	49.6	3951	24	1975.5	987.75	6	987.75	6	987.75	6	987.75	6	50693.8	0.006	1.5	Comm./Ind.	0.048	D	0.83	1	43.5	1.9	77.8	27.2	30
	16	9.0	1225	6	612.5	306.25	1.5	306.25	1.5	306.25	1.5	306.25	1.5	17503.6	0.005	1.5	Comm./Ind.	1.048	D	0.83	1	1895.5	0.1	0.5	0.2	24
1	27,26,25,24,23,22,21,20,19,18,16,17'	161.1	7855	50	3927.5	1963.75	12.5	1963.75	12.5	1963.75	12.5	1963.75	12.5	98454.2	0.006	1.5	Comm./Ind.	0.048	D	0.83	1	76.6	1.2	166.1	58.1	42

Notes: 1) \* = Discharge Point

#### Project: Tucson International Airport Subject: Hydrograph Calculations Author: CGC Location: Tucson, Arizona Date: 11/21/2017

Reference:

Standards Manual for Drainage Design and Floodplain Management In Tucson, Arizona; July 1998 (eff).

Drainage Area ID	Area (acres)	Tc100 (minutes)	i100 (in/hr)	Qp100 (cfs)	Qp5 (cfs)
(PROP) AF-DA-1	41.8	26.7	2.6	78.4	27.4
(PROP) AF-DA-2	160.9	40.4	2.0	271.6	95.1
(PROP) AF-DA-3	77.3	44.6	1.9	115.0	40.2
(PROP) AF-DA-4	588.8	98.5	1.0	495.0	173.3
(PROP) AF-DA-5	623.1	136.4	0.8	387.3	135.6
(PROP) AF-DA-6	64.8	38.2	2.1	114.9	40.2
(EX) AF-DA-1	41.8	26.7	2.6	78.4	27.4
(EX) AF-DA-2	160.9	40.4	2.0	271.6	95.1
(EX) AF-DA-3	77.3	44.6	1.9	115.0	40.2
(EX) AF-DA-4	618.7	102.2	1.0	474.0	165.9
(EX) AF-DA-5	593.3	140.6	0.7	341.3	119.5
(EX) AF-DA-6	64.8	38.2	2.1	114.9	40.2

(EX	) AF-DA-4 (5	-yr)	
t	t (Tr)	Q	Q
factor	minutes	factor	cfs
0	0.0	0	0.0
0.1	7.3	0.025	4.1
0.2	14.5	0.087	14.4
0.3	21.8	0.16	26.5
0.4	29.1	0.243	40.3
0.5	36.4	0.346	57.4
0.6	43.6	0.451	74.8
0.7	50.9	0.576	95.6
0.8	58.2	0.738	122.4
0.9	65.4	0.887	147.2
1	72.7	1.000	165.9
1.1	80.0	0.924	153.3
1.2	87.2	0.839	139.2
1.3	94.5	0.756	125.4
1.4	101.8	0.678	112.5
1.5	109.1	0.604	100.2
1.6	116.3	0.545	90.4
1.7	123.6	0.482	80.0
1.8	130.9	0.424	70.3
1.9	138.1	0.372	61.7
2	145.4	0.323	53.6
2.2	159.9	0.241	40.0
2.4	174.5	0.179	29.7
2.6	189.0	0.136	22.6
2.8	203.6	0.102	16.9
3	218.1	0.078	12.9
3.4	247.2	0.049	8.1
3.8	276.3	0.03	5.0
4.2	305.3	0.02	3.3
4.6	334.4	0.012	2.0
5	363.5	0.008	1.3
7	508.9	0	0.0

	(EX) AF-D	A-5 (5-yr)	
t	t (Tr)	Q	Q
factor	minutes	factor	cfs
0	0.0	0	0.0
0.1	10.7	0.025	3.0
0.2	21.4	0.087	10.4
0.3	32.1	0.16	19.1
0.4	42.8	0.243	29.0
0.5	53.6	0.346	41.3
0.6	64.3	0.451	53.9
0.7	75.0	0.576	68.8
0.8	85.7	0.738	88.2
0.9	96.4	0.887	106.0
1	107.1	1.000	119.5
1.1	117.8	0.924	110.4
1.2	128.5	0.839	100.3
1.3	139.2	0.756	90.3
1.4	149.9	0.678	81.0
1.5	160.7	0.604	72.2
1.6	171.4	0.545	65.1
1.7	182.1	0.482	57.6
1.8	192.8	0.424	50.7
1.9	203.5	0.372	44.5
2	214.2	0.323	38.6
2.2	235.6	0.241	28.8
2.4	257.0	0.179	21.4
2.6	278.5	0.136	16.3
2.8	299.9	0.102	12.2
3	321.3	0.078	9.3
3.4	364.1	0.049	5.9
3.8	407.0	0.03	3.6
4.2	449.8	0.02	2.4
4.6	492.7	0.012	1.4
5	535.5	0.008	1.0
7	749.7	0	0.0

(EX) AF-DA-4 (100-yr)										
t	t (Tr)	Q	Q							
factor	minutes	factor	cfs							
0	0.0	0	0.0							
0.1	7.3	0.025	11.9							
0.2	14.5	0.087	41.2							
0.3	21.8	0.16	75.8							
0.4	29.1	0.243	115.2							
0.5	36.4	0.346	164.0							
0.6	43.6	0.451	213.8							
0.7	50.9	0.576	273.0							
0.8	58.2	0.738	349.8							
0.9	65.4	0.887	420.4							
1	72.7	1.000	474.0							
1.1	80.0	0.924	438.0							
1.2	87.2	0.839	397.7							
1.3	94.5	0.756	358.3							
1.4	101.8	0.678	321.4							
1.5	109.1	0.604	286.3							
1.6	116.3	0.545	258.3							
1.7	123.6	0.482	228.5							
1.8	130.9	0.424	201.0							
1.9	138.1	0.372	176.3							
2	145.4	0.323	153.1							
2.2	159.9	0.241	114.2							
2.4	174.5	0.179	84.8							
2.6	189.0	0.136	64.5							
2.8	203.6	0.102	48.3							
3	218.1	0.078	37.0							
3.4	247.2	0.049	23.2							
3.8	276.3	0.03	14.2							
4.2	305.3	0.02	9.5							
4.6	334.4	0.012	5.7							
5	363.5	0.008	3.8							
7	508.9	0	0.0							

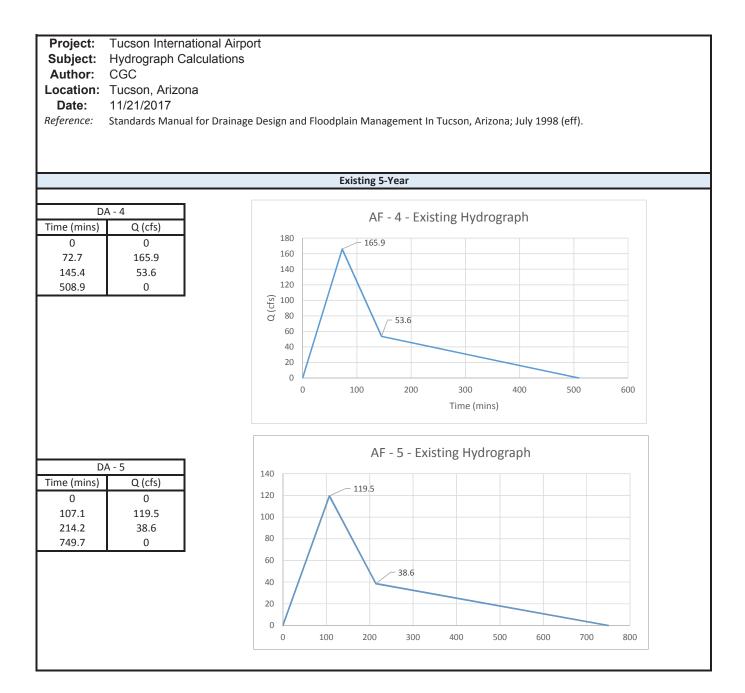
	(EX) AF-DA	-5 (100-yr)	
t	t (Tr)	Q	Q
factor	minutes	factor	cfs
0	0.0	0	0.0
0.1	10.7	0.025	8.5
0.2	21.4	0.087	29.7
0.3	32.1	0.16	54.6
0.4	42.8	0.243	82.9
0.5	53.6	0.346	118.1
0.6	64.3	0.451	153.9
0.7	75.0	0.576	196.6
0.8	85.7	0.738	251.9
0.9	96.4	0.887	302.7
1	107.1	1.000	341.3
1.1	117.8	0.924	315.4
1.2	128.5	0.839	286.4
1.3	139.2	0.756	258.0
1.4	149.9	0.678	231.4
1.5	160.7	0.604	206.1
1.6	171.4	0.545	186.0
1.7	182.1	0.482	164.5
1.8	192.8	0.424	144.7
1.9	203.5	0.372	127.0
2	214.2	0.323	110.2
2.2	235.6	0.241	82.3
2.4	257.0	0.179	61.1
2.6	278.5	0.136	46.4
2.8	299.9	0.102	34.8
3	321.3	0.078	26.6
3.4	364.1	0.049	16.7
3.8	407.0	0.03	10.2
4.2	449.8	0.02	6.8
4.6	492.7	0.012	4.1
	535.5	0.008	2.7
5 7	749.7	0	0.0

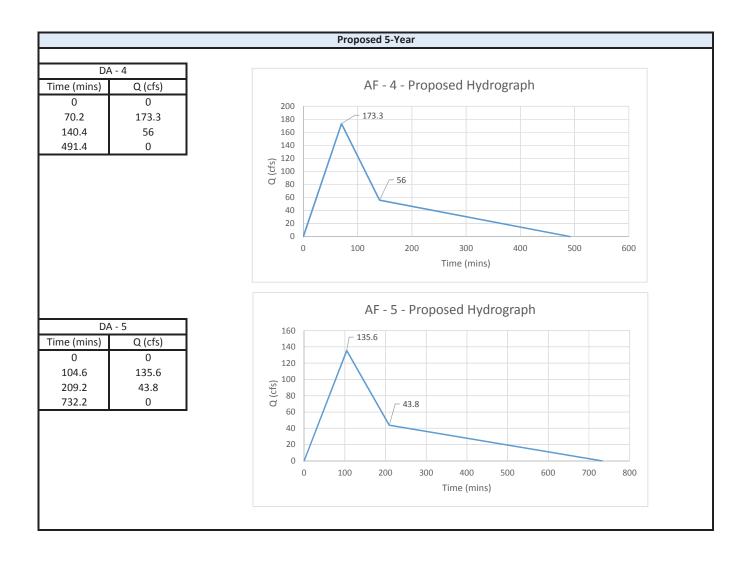
	(PROP) AF-	DA-4 (5-yr)	
t	t (Tr)	Q	Q
factor	minutes	factor	cfs
0	0.0	0	0.0
0.1	7.0	0.025	4.3
0.2	14.0	0.087	15.1
0.3	21.1	0.16	27.7
0.4	28.1	0.243	42.1
0.5	35.1	0.346	60.0
0.6	42.1	0.451	78.2
0.7	49.1	0.576	99.8
0.8	56.2	0.738	127.9
0.9	63.2	0.887	153.7
1	70.2	1.000	173.3
1.1	77.2	0.924	160.1
1.2	84.2	0.839	145.4
1.3	91.3	0.756	131.0
1.4	98.3	0.678	117.5
1.5	105.3	0.604	104.7
1.6	112.3	0.545	94.4
1.7	119.3	0.482	83.5
1.8	126.4	0.424	73.5
1.9	133.4	0.372	64.5
2	140.4	0.323	56.0
2.2	154.4	0.241	41.8
2.4	168.5	0.179	31.0
2.6	182.5	0.136	23.6
2.8	196.6	0.102	17.7
3	210.6	0.078	13.5
3.4	238.7	0.049	8.5
3.8	266.8	0.03	5.2
4.2	294.8	0.02	3.5
4.6	322.9	0.012	2.1
5	351.0	0.008	1.4
7	491.4	0	0.0

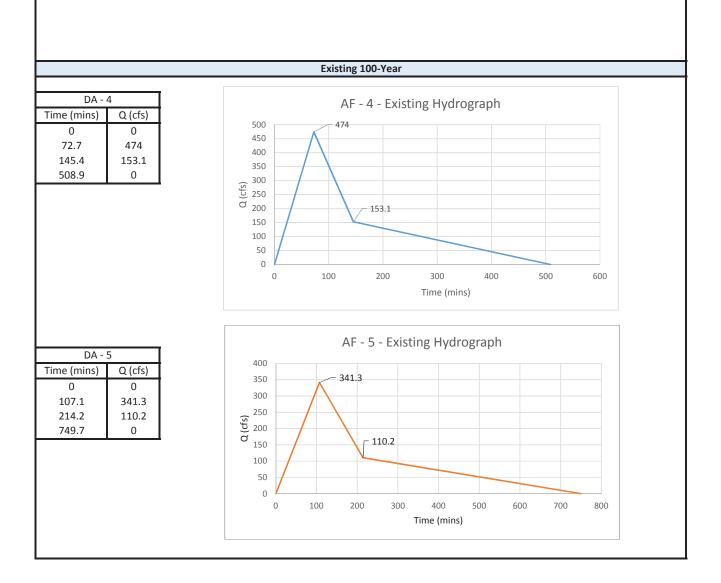
	(PROP) AF-	DA-5 (5-yr)	
t	t (Tr)	Q	Q
factor	minutes	factor	cfs
0	0.0	0	0.0
0.1	10.5	0.025	3.4
0.2	20.9	0.087	11.8
0.3	31.4	0.16	21.7
0.4	41.8	0.243	33.0
0.5	52.3	0.346	46.9
0.6	62.8	0.451	61.2
0.7	73.2	0.576	78.1
0.8	83.7	0.738	100.1
0.9	94.1	0.887	120.3
1	104.6	1.000	135.6
1.1	115.1	0.924	125.3
1.2	125.5	0.839	113.8
1.3	136.0	0.756	102.5
1.4	146.4	0.678	91.9
1.5	156.9	0.604	81.9
1.6	167.4	0.545	73.9
1.7	177.8	0.482	65.4
1.8	188.3	0.424	57.5
1.9	198.7	0.372	50.4
2	209.2	0.323	43.8
2.2	230.1	0.241	32.7
2.4	251.0	0.179	24.3
2.6	272.0	0.136	18.4
2.8	292.9	0.102	13.8
3	313.8	0.078	10.6
3.4	355.6	0.049	6.6
3.8	397.5	0.03	4.1
4.2	439.3	0.02	2.7
4.6	481.2	0.012	1.6
5	523.0	0.008	1.1
7	732.2	0	0.0

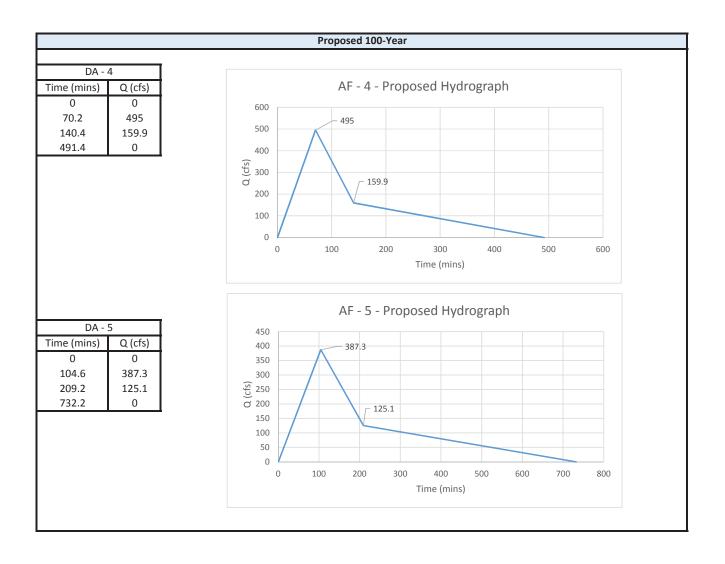
	PROP) AF-D	A-4 (100-y	r)
t	t (Tr)	Q	Q
factor	minutes	factor	cfs
0	0.0	0	0.0
0.1	7.0	0.025	12.4
0.2	14.0	0.087	43.1
0.3	21.1	0.16	79.2
0.4	28.1	0.243	120.3
0.5	35.1	0.346	171.3
0.6	42.1	0.451	223.2
0.7	49.1	0.576	285.1
0.8	56.2	0.738	365.3
0.9	63.2	0.887	439.1
1	70.2	1.000	495.0
1.1	77.2	0.924	457.4
1.2	84.2	0.839	415.3
1.3	91.3	0.756	374.2
1.4	98.3	0.678	335.6
1.5	105.3	0.604	299.0
1.6	112.3	0.545	269.8
1.7	119.3	0.482	238.6
1.8	126.4	0.424	209.9
1.9	133.4	0.372	184.1
2	140.4	0.323	159.9
2.2	154.4	0.241	119.3
2.4	168.5	0.179	88.6
2.6	182.5	0.136	67.3
2.8	196.6	0.102	50.5
3	210.6	0.078	38.6
3.4	238.7	0.049	24.3
3.8	266.8	0.03	14.9
4.2	294.8	0.02	9.9
4.6	322.9	0.012	5.9
5	351.0	0.008	4.0
7	491.4	0	0.0

(PROP) AF-DA-5 (100-yr)					
t t(Tr) Q Q					
factor	minutes	factor	cfs		
0	0.0	0	0.0		
0.1	10.5	0.025	9.7		
0.2	20.9	0.087	33.7		
0.3	31.4	0.16	62.0		
0.4	41.8	0.243	94.1		
0.5	52.3	0.346	134.0		
0.6	62.8	0.451	174.7		
0.7	73.2	0.576	223.1		
0.8	83.7	0.738	285.8		
0.9	94.1	0.887	343.5		
1	104.6	1.000	387.3		
1.1	115.1	0.924	357.9		
1.2	125.5	0.839	324.9		
1.3	136.0	0.756	292.8		
1.4	146.4	0.678	262.6		
1.5	156.9	0.604	233.9		
1.6	167.4	0.545	211.1		
1.7	177.8	0.482	186.7		
1.8	188.3	0.424	164.2		
1.9	198.7	0.372	144.1		
2	209.2	0.323	125.1		
2.2	230.1	0.241	93.3		
2.4	251.0	0.179	69.3		
2.6	272.0	0.136	52.7		
2.8	292.9	0.102	39.5		
3	313.8	0.078	30.2		
3.4	355.6	0.049	19.0		
3.8	397.5	0.03	11.6		
4.2	439.3	0.02	7.7		
4.6	481.2	0.012	4.6		
5	523.0	0.008	3.1		
7	732.2	0	0.0		









Project: Tucson International Airport Subject: Soils Summary Author: CGC Location: Tucson, Arizona Date: 11/21/2017

*Reference:* Standards Manual for Drainage Design and Floodplain Management In Tucson, Arizona; July 1998 (eff).

MUSYM	SCS Designation (B,C,D)	
11	D	
34	С	
72	С	
78	A (B used)	
86	С	
CIC	D	
LeB	В	
LrB	В	
MhB	С	
ReC	В	



Tucson International Airport

Conceptual Drainage Design

# Appendix A.3 – Airfield C-Values Calculations

DA	Cw	Total Area (acres)		
Existing				
1	0.7	41.8		
2	0.85	160.9		
3	0.79	77.3		
4	0.8	618.7		
5	0.79	593.3		
6	0.86	64.8		
Proposed				
1	0.73	41.8		
2	0.85	160.9		
3	0.8	77.3		
4	0.83	588.8		
5	0.81	623.1		
6	0.86	64.8		

DA	Area (sf)	Area (acres)		
Existing				
1	1,820,116	41.8		
2	7,007,424	160.9		
3	3,368,198	77.3		
4	26,952,645	618.7		
5	25,845,028	593.3		
6	2,823,329	64.8		
Proposed				
1	1,820,116	41.8		
2	7,007,424	160.9		
3	3,368,198	77.3		
4	25,650,188	588.8		
5	27,142,971	623.1		
6	2,823,329	64.8		

DA - Existing	MUSYM	LU_Type	Pervious Area (acres)	SCS Designation	c	Total Pervious Area (acres)	Total Impervious Area (acres)	C*A Pervious	Cw Pervious	Cw Impervious	Weighted C-Value
	11 11 11	15 15 15	0.004 0.143 0.332	D D D	0.72 0.72 0.72			0.0 0.1 0.2			
	11 11 11	15 15 15	0.503 0.656	D	0.72 0.72 0.72			0.4 0.5			
	11	15 15 15	2.337	D	0.72			1.7			
1	11 11	5	2.964 0.004	D	0.72 0.69	32.6	9.2	2.1	0.67	0.95	0.73
	11 72	5 15	11.429 0.558	D	0.69			7.9			
	72 72	15 15	0.766	C C	0.64			0.5			
	72 72	15 5	1.660 7.918	C C	0.64 0.61			1.1 4.8			
	86 86	15 5	0.473 1.696	C C	0.64 0.61			0.3 1.0			
	11 11	15 15	0.020 0.020	D	0.72 0.72			0.0			
	11 11	15 15	1.127 1.210	D	0.72 0.72			0.8 0.9			
	11 11	15 15	1.236 1.290	D	0.72 0.72			0.9 0.9			
	11 11	15 5	12.409 3.848	D	0.72 0.69			8.9 2.7			
	72 72	15 15	0.308 0.618	C C	0.64 0.64			0.2			
	72 72	15 15	0.785	c c	0.64 0.64			0.5			
2	72 72	15 15	1.715 1.892	c c	0.64 0.64	54.5	106.4	1.1 1.2	0.66	0.95	0.85
	72 72	15 15	2.186 2.403	C C	0.64 0.64			1.4 1.5			
	72 72	15 15	2.896 6.625	C C	0.64 0.64			1.9 4.2			
	72 72	5	0.072 0.196	c	0.61 0.61			0.0 0.1			
	72 72	5	2.160 2.547	c c	0.61 0.61			1.3 1.6			
	86 86	15 15	0.789 2.487	C C	0.64			0.5			
	86	5	4.578	с с	0.61			2.8			
	72 72 72	15 15 15	0.855	c	0.64			0.5			
	72 72	15 15 15	2.951	c	0.64			1.9			
3	72	15	3.605 7.747	c	0.64 0.64	36.0	41.3	2.3	0.63	0.95	0.80
	72 72	15 5	12.045 4.946	C C	0.61			7.7			
	LrB ReC	15 15	0.204 2.257	B B	0.54 0.54			0.1 1.2			
	11 11	15 15	0.000 0.003	D	0.72 0.72			0.0 0.0			
	11 11	15 15	1.973 2.244	D	0.72			1.4 1.6			
	11 11	15 15	2.370 4.298	D	0.72 0.72			1.7 3.1			
	11 11	15 15	8.485 12.952	D	0.72			6.1 9.3			
	11 11	15 15	16.874 18.021	D	0.72 0.72			12.1 13.0			
	11 11	5	0.000 0.001	D	0.69 0.69			0.0			
	11 11	5	0.001 0.003	D	0.69 0.69			0.0			
	11 11	5	0.071 0.321	D	0.69 0.69			0.0			
	11 11	5	0.562 3.624	D	0.69 0.69			0.4 2.5			
	11 11	5	6.190 6.685	D	0.69 0.69			4.3 4.6			
	11 72	5 15	24.617 0.001	D C	0.69 0.64			17.0 0.0			
	72 72	15 15	0.039 0.184	C C	0.64 0.64			0.0			
	72 72	15 15	0.892 1.019	C C	0.64 0.64			0.6 0.7			
	72 72	15 15	1.701 1.962	C C	0.64			1.1 1.3			
	72 72	15 15	2.211 2.423	c	0.64			1.4			
	72 72	15 15	2.747 3.231	c c	0.64 0.64			1.8 2.1			
	72 72	15 15	6.778 9.774	c c	0.64 0.64			4.3 6.3			
4	72 72	15 15	18.340 19.444	C C	0.64 0.64	347.3	271.4	11.7 12.4	0.65	0.95	0.78
	72 72	15 5	43.110 0.813	C C	0.64			27.6 0.5			
	72 72	5	1.469 2.170	C C	0.61			0.9			
	72 72	5	2.541 4.110	C C	0.61			1.6			
	72 72 72	5	10.248 11.933	c	0.61			6.3 7.3			
	72 72 72	5	13.043 15.695	c	0.61			8.0 9.6			
	86 86	15 15	0.003 0.008	c	0.64			0.0			
	86 86	15 15 15	0.015 0.232	c	0.64			0.0			
	86 86	15 15 15	0.436 0.684	c	0.64			0.3 0.4			
	86 86	15 15 15	0.684 5.242 5.780	c	0.64 0.64 0.64			0.4 3.4 3.7			
	86 86	15 15 5	5.780 11.561 0.018	c	0.64 0.61			3.7 7.4 0.0			
	86 86	5	0.018 0.334 0.491	C	0.61 0.61 0.61			0.0 0.2 0.3			
	86	5	1.261	C C	0.61			0.8			
	86 86	5	3.028 3.060	C C	0.61			1.8			
	86 86	5	5.408 8.476	C C	0.61 0.61			3.3 5.2			
	86 CIC	5 15	10.311 1.934	C 0	0.61 0.72			6.3 1.4			
	CIC LeB	15 15	2.422 0.164	0 B	0.72 0.54			1.7 0.1			
	LeB MhB	15 15	0.355 0.694	B C	0.54 0.64			0.2			
	ReC	15	0.247	в	0.54			0.1			

11         15         0.003         D         0.72         0.0           11         15         0.286         D         0.72         0.2	2
11 15 0.360 D 0.72 0.3	3
11 15 0.429 D 0.72 0.3 11 15 0.338 D 0.72 0.7	7
11 15 1.637 D 0.72 1.2	2
11         15         1.942         D         0.72         1.4           11         15         2.265         D         0.72         1.6	
11 15 3.354 D 0.72 24	
11 15 5521 D 0.72 40	
11 15 6.175 D 0.72 44 11 15 13.131 D 0.72 95	
11 15 18.624 D 0.72 13.	4
11 5 0.000 D 0.69 0.00 11 5 0.006 D 0.69 0.00	
11 5 0.035 D 0.69 0.0	
11         5         0.104         D         0.69         0.1           11         5         0.136         D         0.69         0.1	
11 5 0.256 D 0.69 0.7	
11 5 0.269 D 0.69 0.2	2
11         5         0.333         D         0.69         0.4           11         5         0.708         D         0.69         0.5	
11 5 1.307 D 0.69 0.9	
11         5         1.325         D         0.69         0.5           11         5         1.509         D         0.69         1.0	
11 5 1.812 D 0.69 1.3	3
11 5 2.740 D 0.69 15. 11 5 5.559 D 0.69 33.	
11 5 5.639 D 0.69 5.3 11 5 6.088 D 0.69 4.7	
11 5 6.478 D 0.69 4.5	
11 5 6.651 D 0.69 4.4 11 5 8.137 D 0.69 5.6	
11 5 12.156 D 0.69 8.4	1
11 5 12.833 D 0.69 8.8.1 11 5 72.844 D 0.669 15	
11 5 21.844 D 0.69 15. 11 5 37.183 D 0.69 25.	
72 15 0.001 C 0.64 0.0	)
72         15         0.001         C         0.64         0.0           72         15         0.005         C         0.64         0.0	
72 15 0.138 C 0.64 0.1	L
72 15 0.216 C 0.64 0.1 72 15 0.325 C 0.64 0.2	
72 15 0.428 C 0.64 0.3	
72 15 0.578 C 0.64 04 72 15 1.204 C 0.64 68	
72 15 1.204 C 0.04 0. 72 15 1.474 C 0.64 0.5	
5 72 15 1.664 C 0.64 350.7 242.7 1.1	
72 15 3,488 C 0.64 22.7 72 15 3,615 C 0.64 2.7	
72 15 7.587 C 0.64 4.5	
72 15 8.878 C 0.64 5.7 72 5 0.000 C 0.61 0.01	
72 5 0.000 C 0.61 0.00 72 5 0.001 C 0.61 0.00	
72 5 0.001 C 0.61 0.0	
72         5         0.005         C         0.61         0.0           72         5         0.015         C         0.61         0.0	
72 5 0.062 C 0.61 0.0	)
72 5 0.079 C 0.61 00 72 5 0.079 C 0.61 0.00	
72 5 0.193 C 0.61 0.1	
72 5 0.280 C 0.61 0.2	2
72 5 0.377 C 0.61 0.27 72 5 1.169 C 0.61 0.77	7
72 5 3.366 C 0.61 2.1	L
72 5 5.183 C 0.61 3.1 72 5 5.283 C 0.61 3.1	
72 5 10.040 C 0.61 6.1	L
78 15 7.948 8 0.54 4.3 78 5 0.244 8 0.49 0.1	
78 5 0.834 B 0.49 0.4	1
78 5 16.411 8 0.49 8.3. 86 15 0.001 C 0.64 0.0	
86 15 0.008 C 0.64 0.0	
86 15 0.015 C 0.64 0.0	
86 15 0.052 C 0.64 0.0 86 15 0.738 C 0.64 0.5	
86 15 0.988 C 0.64 0.6	5
86         15         2.153         C         0.64         1.4           86         15         2.257         C         0.64         1.4	
86 15 7.743 C 0.64 5.0	
86 15 13.976 C 0.64 8.5 86 5 0.001 C 0.61 0.0	
86 5 0.001 C 0.61 0.0 86 5 0.001 C 0.61 0.0	
86 5 0.174 C 0.61 0.1	L
86 5 0.212 C 0.61 0.1 86 5 0.290 C 0.61 0.2	
86 5 1.560 C 0.61 1.0	)
86 5 1.609 C 0.61 1.1 86 5 1.816 C 0.61 1.1	
86 5 1.816 C 0.61 1.1 86 5 1.955 C 0.61 1.2	
86 5 2.517 C 0.61 1.5	
86 5 10.812 C 0.61 6.6 86 5 17.964 C 0.61 111	
86 5 20.383 C 0.61 12.	4
11         5         0.000         D         0.69         0.0           11         5         0.000         D         0.69         0.0	
11 5 0.001 D 0.69 0.0	
11 5 0.001 D 0.69 0.0 11 5 0.003 D 0.69 0.0	
	5
11 5 0.857 D 0.69 0.6	
6 11 5 5.435 D 0.69 27 421 3.8	0.09 0.95 0.00
6 11 5 5.435 D 0.69 22.7 42.1 3.8 11 5 15.088 D 0.69 10.69 10.00	
6         11         5         5.435         D         0.69         22.7         42.1         3.8           11         5         15.088         D         0.69         20.7         42.1         10.           72         5         0.000         C         0.61         0.0           72         5         0.000         C         0.61         0.0	4 ) )
6         11         5         5.835         D         0.69         22.7         42.1         3.8           11         5         15.088         D         0.69         22.7         42.1         10.0           72         5         0.000         C         0.61         0.0           72         5         0.000         C         0.61         0.0           72         5         0.001         C         0.61         0.0	4 ) )
6         11         5         5.435         D         0.69         22.7         42.1         3.8           11         5         15.088         D         0.69         20.7         42.1         10.           72         5         0.000         C         0.61         0.0           72         5         0.000         C         0.61         0.0	4 ) ) ) }

DA - Proposed	MUSYM	LU_Type	Pervious Area (acres)	SCS Designation	с	Total Pervious Area (acres)	Total Impervious Area (acres)	C*A Pervious	Cw Pervious	Cw Impervious	Weighted C-Value
	11	5	11.429	D	0.69			7.89			
	11 11	15 15	0.656 0.332	D D	0.72			0.47			
	11	15	2.964	D	0.72			2.13			
	11 11	15 15	0.503 0.143	D	0.72			0.36 0.10			
	11	15	2.337	D	0.72			1.68			
1	72 72	5 15	7.918 0.558	C C	0.61 0.64	32.6	9.2	4.83 0.36	0.67	0.95	0.73
	72	15	1.137	C	0.64			0.73			
	72 72	15 15	0.766	C C	0.64			0.49 1.06			
	86 86	5 15	1.696 0.473	c c	0.61			1.03 0.30			
	11	5	0.004	D	0.69			0.00			
	11	15	0.004	D	0.72			0.00			
	11	15	1.236	D	0.72			0.89			
	11 11	15 15	1.210 12.409	D D	0.72			0.87 8.93			
	11	15	0.020	D	0.72			0.01			
	11 72	15 5	0.020 0.196	D C	0.72 0.61			0.01 0.12			
	72 72	5	0.072 2.160	C C	0.61 0.61			0.04			
	72	5	2.547	C	0.61			1.55			
	72 72	15 15	1.052 2.896	C C	0.64			0.67 1.85			
2	72	15	0.785	C	0.64	54.5	106.4	0.50	0.66	0.95	0.85
	72 72	15 15	0.308 0.618	C C	0.64			0.20 0.40			
	72	15 15	1.715	c	0.64			1.10			
	72	15	6.625 2.186	С	0.64			4.24 1.40			
	72 72	15 15	2.403 1.892	c c	0.64 0.64			1.54 1.21			
	11	5	3.848	D	0.69			2.66			
	11 86	15 5	1.290 4.578	D C	0.72 0.61			0.93 2.79			
	86	15	0.789	C	0.64			0.50			
	86 ReC	15	2.487 2.257	C B	0.64			1.59			
	LrB 72	15 5	0.204	B C	0.54			0.11 3.02			
	72	15	12.045	С	0.64			7.71			
3	72 72	15 15	0.377 1.063	C C	0.64 0.64	36.0	41.3	0.24 0.68	0.63	0.95	0.80
	72	15	2.951	C	0.64			1.89			
	72 72	15 15	7.747 3.605	c c	0.64 0.64			4.96 2.31			
	72 MhB	15 15	0.855	С	0.64			0.55			
	LeB	15	0.694 0.355	C B	0.64 0.54			0.19			
	LeB CIC	15 15	0.164 2.422	B	0.54 0.72			0.09 1.74			
	CIC	15	1.934	0	0.72			1.39			
	ReC 11	15 60	0.247 2.729	B D	0.54			0.13 2.29			
	11	60	1.469	D	0.84			1.23			
	11 11	60 60	3.230 1.818	D	0.84			2.71 1.53			
	11 11	60 60	9.442 2.503	D D	0.84			7.93 2.10			
	11	60	9.412	D	0.84			7.91			
	11 11	60 60	7.881 0.112	D D	0.84			6.62 0.09			
	72	5	13.043	C	0.61			7.96			
	72 72	5	4.196 11.933	C C	0.61			2.56 7.28			
	72 72	5 15	1.469 29.254	C C	0.61			0.90 18.72			
	72	15	9.774	C	0.64			6.26			
	72 72	15 15	6.778 1.701	C C	0.64			4.34 1.09			
	72	15	0.892	С	0.64			0.57			
	72 72	15 15	2.194 1.962	C C	0.64 0.64			1.40 1.26			
	72 72	15 15	3.231 0.453	c c	0.64 0.64			2.07 0.29			
	72	15	0.184	c	0.64			0.12			
	72 72	15 60	0.039 2.798	C C	0.64			0.02 2.27			
	72	60	3.232	С	0.81			2.62			
	72 72	60 60	2.900 1.456	C C	0.81 0.81			2.35 1.18			
	72 72	60 60	16.058 3.369	c c	0.81 0.81			13.01 2.73			
4	72	60	0.099	C	0.81	294.6	294.3	0.08	0.71	0.95	0.83
	72 72	60 60	2.638 12.618	C C	0.81			2.14 10.22			
	72	60 5	9.269 3.624	C D	0.81			7.51			
	11	5	0.071	D	0.69			2.50 0.05			
	11 11	15 15	1.973 8.485	D D	0.72			1.42 6.11			
	11	5	24.617	D	0.69			16.99			
	11 11	5	0.562 6.685	D	0.69			0.39 4.61			
	11	5	6.190	D	0.69			4.27			
	11 72	15 5	0.000 0.813	D C	0.72 0.61			0.00			
	72 72	5	4.110 2.541	c c	0.61 0.61			2.51 1.55			
	11	5	0.321	D	0.69			0.22			
	86 86	5 15	3.332 0.684	C C	0.61 0.64			2.03 0.44			
	86	15	0.232	C	0.64			0.15			
	86 86	15 15	5.242 0.866	c c	0.64 0.64			3.35 0.55			
	86 86	60	15.152	C	0.81			12.27			
	86	5 5	0.334 0.491	C C	0.61			0.20 0.30			
	86 86	5 5	8.476 3.060	C C	0.61 0.61			5.17 1.87			
	86	5	10.311	C	0.61			6.29			
	86 86	5 15	0.018 0.436	C C	0.61			0.01 0.28			
	86	15	0.003	c	0.64			0.00			
			0.001	~							
	72 11	5	0.001 0.001	C D	0.61 0.69			0.00			
	72	5									

	11	5	6.478	D	0.69			4.47			
	11 11	5 60	1.325 3.690	D D	0.69 0.84			0.91 3.10			
	11	60	8.159	D	0.84			6.85			
	11 11	60 60	8.402 9.723	D D	0.84 0.84			7.06 8.17			
	11	60	0.230	D	0.84			0.19			
	11	60	5.657	D	0.84			4.75			
	72 72	5 5	0.013 4.004	C C	0.61 0.61			0.01 2.44			
	72	5	0.512	c	0.61			0.31			
	72	5	0.062	C	0.61			0.04			
	72 72	15 60	1.369 12.013	c c	0.64 0.81			0.88 9.73			
	72	60	0.383	c	0.81			0.31			
	72	60	5.435	C	0.81			4.40			
	72 72	60 60	0.063 4.315	c c	0.81 0.81			0.05 3.50			
	72	60	0.105	c	0.81			0.08			
	11	5	0.104	D	0.69			0.07			
	72 72	5	0.015 0.079	c c	0.61			0.01 0.05			
	72	5	3.366	c	0.61			2.05			
	72	5	0.193	C	0.61			0.12			
	72 72	5 5	5.283 5.183	c c	0.61 0.61			3.22 3.16			
	72	5	0.079	c	0.61			0.05			
	72	5	0.377	с	0.61			0.23			
	72 72	15 15	1.204 0.578	c c	0.64			0.77 0.37			
1	72	15	3.488	C	0.64			2.23			
1	72	15	0.428	C	0.64			0.27			
	72 72	15 15	0.216 0.325	C C	0.64			0.14 0.21			
1	72	15	3.615	C	0.64			2.31			
	11	5	1.509	D	0.69			1.04			
	11 11	5 5	12.156 0.333	D D	0.69			8.39 0.23			
	11	5	6.088	D	0.69			4.20			
	11	5	0.035	D	0.69			0.02			
	11 11	5 5	5.659 1.812	D D	0.69 0.69			3.90 1.25			
1	11	5	0.256	D	0.69			0.18			
	11	5	0.006	D	0.69			0.00			
1	11 11	5 5	7.891 21.844	D D	0.69			5.44 15.07			
	11	5	6.651	D	0.69			4.59			
	11	5	12.833	D	0.69			8.86			
	11 11	5 5	27.423 2.740	D D	0.69 0.69			18.92 1.89			
	11	5	1.307	D	0.69			0.90			
_	11	5	0.136	D	0.69	224.5	200 -	0.09	0.00	0.05	
5	11 11	5 15	0.269 1.637	D D	0.69 0.72	324.8	298.3	0.19 1.18	0.69	0.95	0.81
	11	15	3.354	D	0.72			2.41			
1	11	15	0.286	D	0.72			0.21			
	11 11	15 15	0.360 2.265	D D	0.72 0.72			0.26 1.63			
	11	15	6.175	D	0.72			4.45			
1	11	15	0.938	D	0.72			0.68			
	11 11	60 60	0.419 1.880	D D	0.84 0.84			0.35 1.58			
	78	5	0.244	В	0.49			0.12			
	78	5	16.441	в	0.49			8.06			
1	78 78	5 15	0.834 7.948	B	0.49 0.54			0.41 4.29			
	86	5	2.076	С	0.61			1.27			
	86	5	17.439	c	0.61			10.64			
	86 86	5 5	1.955 1.816	c c	0.61 0.61			1.19 1.11			
	86	5	0.290	C	0.61			0.18			
	86 86	5 5	2.517 3.011	C C	0.61 0.61			1.54 1.84			
	86	5	2.665	c	0.61			1.63			
	86	5	1.609	С	0.61			0.98			
	86 86	5 5	0.559 0.001	C C	0.61 0.61			0.34 0.00			
	86	5	5.273	C	0.61			3.22			
	86	5	0.174	c	0.61			0.11			
	86 86	15 15	7.090 2.257	C C	0.64 0.64			4.54 1.44			
	86	15	0.209	C	0.64			0.13			
	86	15	0.155	с	0.64			0.10			
	86 86	15 15	8.552 0.738	c c	0.64 0.64			5.47 0.47			
	86	60	0.098	C	0.81			0.08			
	86 86	60 60	7.576 0.123	C C	0.81 0.81			6.14 0.10			
	86 86	60 60	0.123	c	0.81			0.10			
	86	60	5.076	С	0.81			4.11			
	86 86	60 60	1.554 1.900	C C	0.81 0.81			1.26 1.54			
	86 86	60 60	1.900 0.009	c	0.81			0.01			
	86	5	1.560	С	0.61			0.95			
	86 72	5 5	0.212 0.001	C C	0.61 0.61			0.13 0.00			
	11	5	0.001	D	0.69			0.00			
	72	5	0.001	C	0.61			0.00			
	72 72	5 5	0.000 0.005	C C	0.61 0.61			0.00			
	72 72	5	0.005	c	0.61			0.00			
	72	15	0.005	C	0.64			0.00			
	72 86	15 5	0.001 0.001	C C	0.64 0.61			0.00			
	86	15	0.001	С	0.64			0.00			
	CIC	15	0.859	0	0.72			0.62			
	11 11	5 5	0.857 5.435	D D	0.69 0.69			0.59 3.75			
	11	5	15.088	D	0.69			10.41			
	72	5	0.000	C	0.61			0.00			
1		5	0.470	C C	0.61 0.61			0.29 0.00			
	72 72	5	0.002			22.7	42.1	0.00		0.05	
6	72 11	5 5	0.002	D	0.69	22.7	42.1	0.00	0.69	0.95	0.86
6	72 11 11	5 5 5	0.000 0.001	D D	0.69 0.69	22.1	42.1	0.00	0.69	0.95	0.86
6	72 11 11 11	5 5 5	0.000 0.001 0.000	D D D	0.69 0.69 0.69	22.7	42.1	0.00 0.00	0.69	0.95	0.86
6	72 11 11 11 11 11	5 5 5 5 5	0.000 0.001 0.000 0.001	D D D	0.69 0.69 0.69 0.69	22.7	42.1	0.00 0.00 0.00	0.69	0.95	0.86
6	72 11 11 11	5 5 5	0.000 0.001 0.000	D D D	0.69 0.69 0.69	22.7	42.1	0.00 0.00	0.69	0.32	0.86



Conceptual Drainage Design

### Appendix B – Basin Sizing

Project:Tucson Int. AirportLocation:Tucson, ArizonaDate:November 21, 2017Subject:Retention Basin SizingContributing Sub-Basins:

#### Proj. Number: 551424 Proj. Engineer: Cole Cooper Checker: Lloyd Vick

1,2,3,4,5,6,7,8,9,10,11,12,13,14,15

	Tucson Int. Airport	- Basin 1	
CONTOUR ELEVATION	AREA	VOLUME PROVIDED (CFT)	VOLUME PROVIDED (Ac-Ft)
2557	25,256	24,070	0.5526
2556	22,884	21,748	0.4993
2555	20,612	0	0.0000
		0	0.0000
		0	0.0000
		0	0.0000
		0	0.0000
			0.0000

Volume Required (ac-ft):
100-yr:
0.8 ac-ft

Total Volume= 45,818 1.1

Note:

1) Average End Area Method Utilized for Calculating Volume Provided

Project:Tucson Int. AirportLocation:Tucson, ArizonaDate:November 21, 2017Subject:Retention Basin SizingContributing Sub-Basins:

Proj. Number: 551424 Proj. Engineer: Cole Cooper Checker: Lloyd Vick

1,2,3,4,5,6,7,8,9,10,11,12,13,14,15

	Tucson Int. Airport - Basin 1						
CONTOUR ELEVATION	AREA	VOLUME PROVIDED	VOLUME PROVIDED				
2595	139,757	(CFT) 136,686	(Ac-Ft) 3.1379				
2595	133,615	130,595	2.9980				
2593	127,575	124,583	2.8600				
2592	121,590	118,666	2.7242				
2591	115,741	0	0.0000				
		0	0.0000				
		0	0.0000				
			0.0000				

Volume Required (ac-ft):
100-yr:
10.8 ac-ft

Total Volume= 510,529 11.7

Note:

1) Average End Area Method Utilized for Calculating Volume Provided



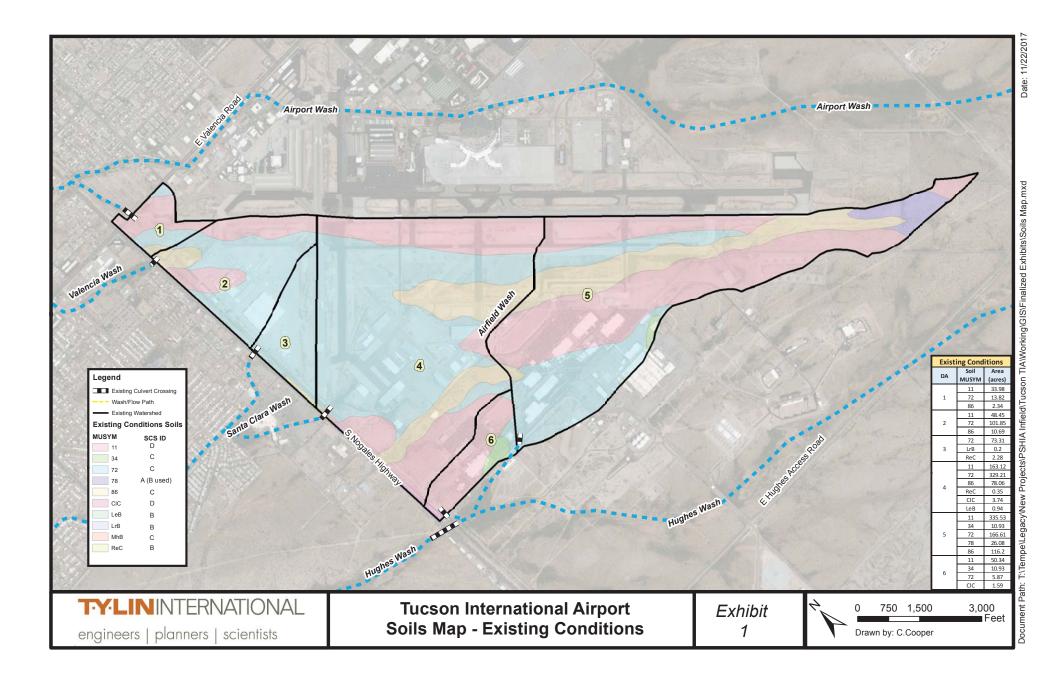
Conceptual Drainage Design

## Appendix C – Exhibits



Conceptual Drainage Design

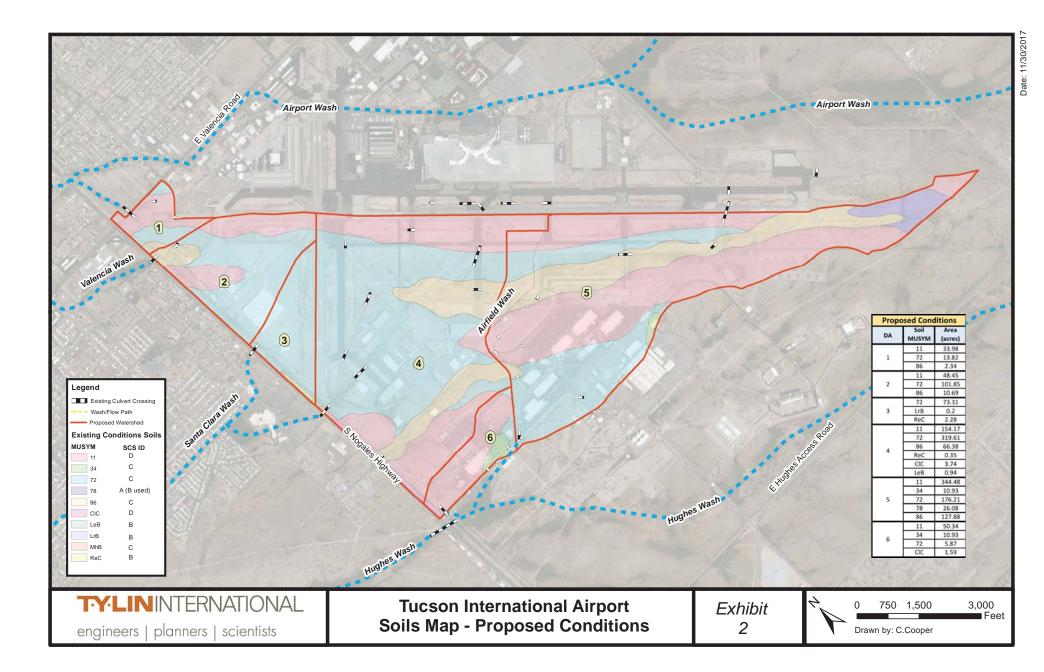
### Appendix C.1 – Exhibit 1 – Soils Map – Existing Conditions





Conceptual Drainage Design

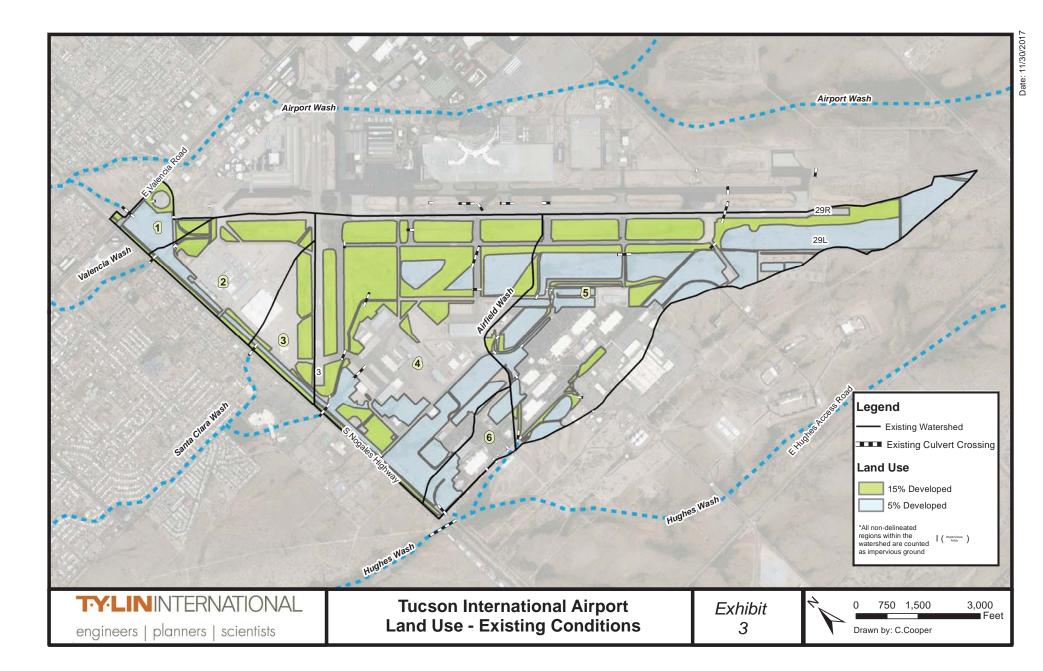
#### **Appendix C.2 – Soils Map – Proposed Conditions**







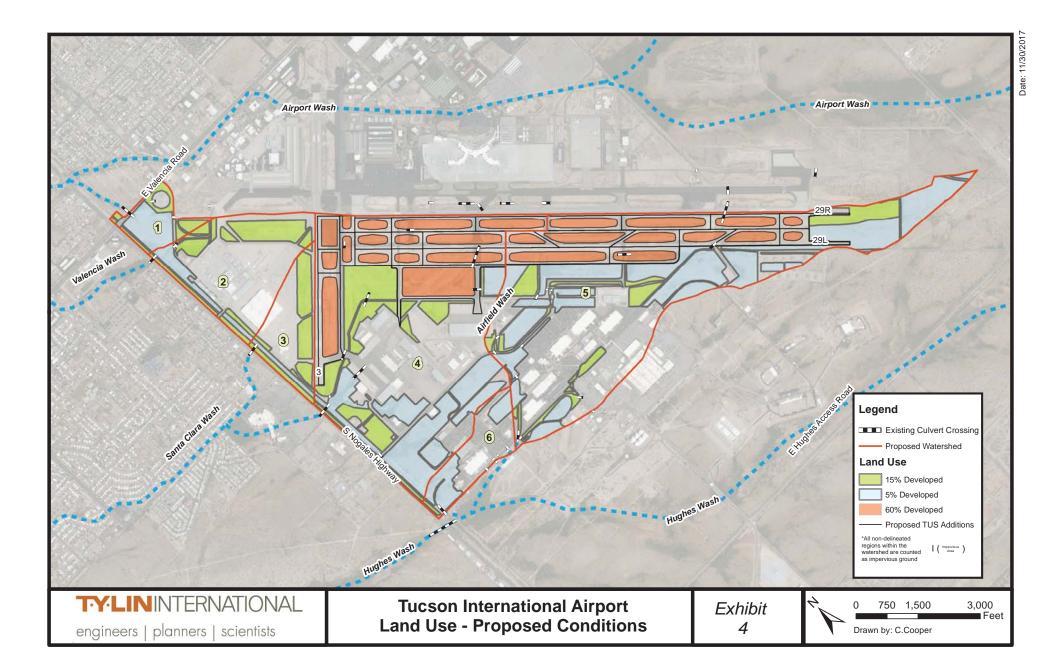
## Appendix C.3 – Land Use – Existing Conditions







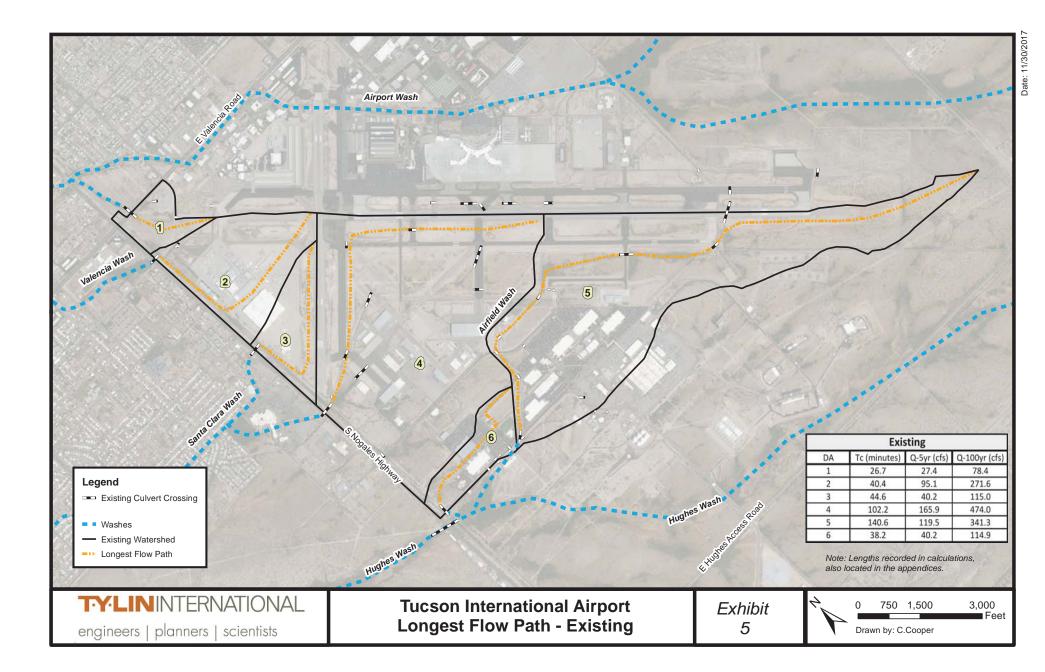
## Appendix C.4 – Land Use – Proposed Conditions

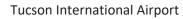






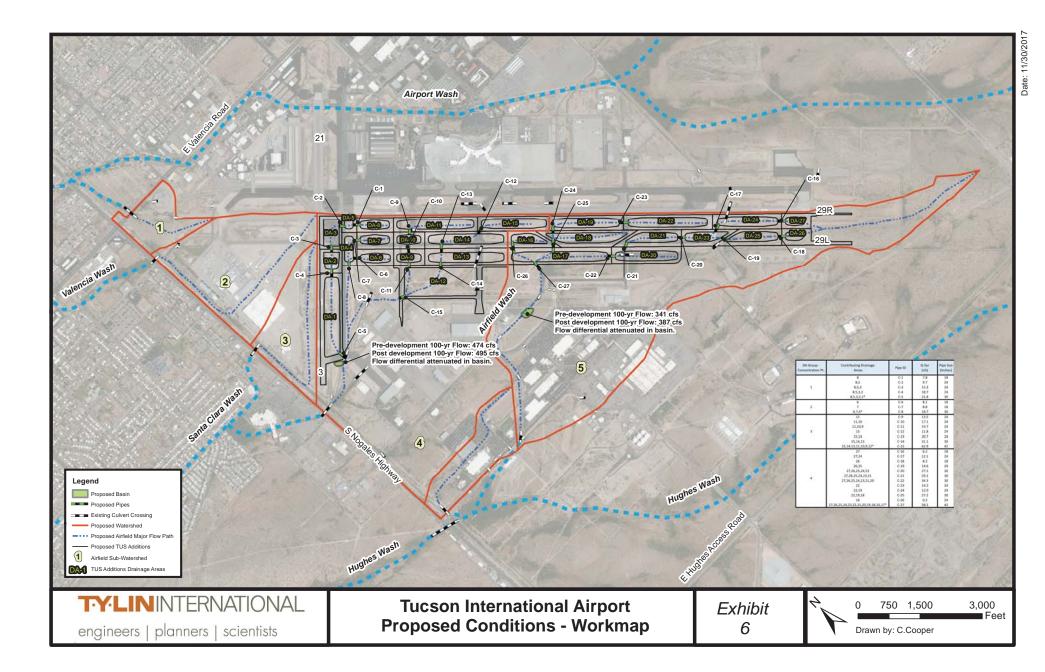
## **Appendix C.5 – Longest Flow Path - Existing**







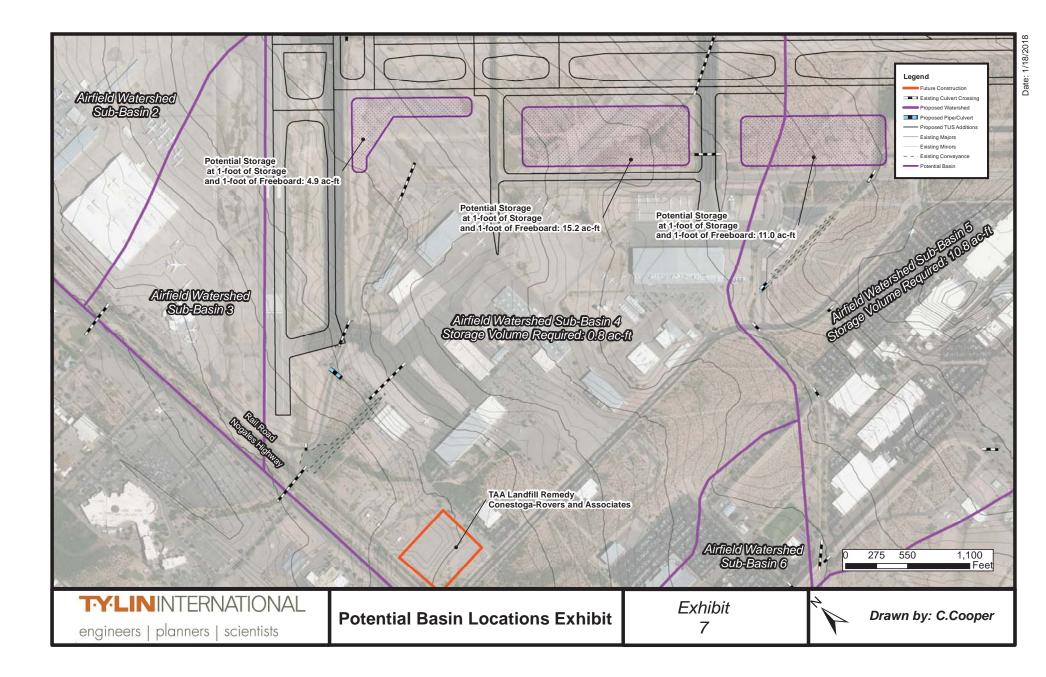
# **Appendix C.6 – Proposed Conditions - Workmap**



Conceptual Drainage Design



## Appendix C.7 – Basin Exhibit





Conceptual Drainage Design

### **Appendix D – FlowMaster (Pipe Sizing)**

#### **Pipe Sizing - TIA Report**

Label	Roughness Coefficient	Channel Slope (ft/ft)	Diameter (ft)
DA - 8	0.013	0.00980	1.50
DA - 8,5	0.013	0.00500	2.00
DA - 8,5,3	0.013	0.00710	2.00
DA - 8,5,3,2	0.013	0.00660	2.00
DA - 8,5,3,2,1	0.013	0.00390	2.50
DA - 6	0.013	0.00600	1.50
DA - 7	0.013	0.00610	1.50
DA - 6,7,4	0.013	0.00540	2.50
DA - 11	0.013	0.00720	2.00
DA - 11,10	0.013	0.00720	2.00
DA - 11,10,9	0.013	0.00840	2.00
DA - 15	0.013	0.00730	2.00
DA - 15,14	0.013	0.00730	2.00
DA - 15,14,13	0.013	0.00680	2.50
DA - 15,14,13,11,10,9,12	0.013	0.00700	3.50
DA - 27	0.013	0.01230	1.50
DA - 27,24	0.013	0.00390	2.00
DA - 26	0.013	0.01230	1.50
DA - 26,25	0.013	0.00440	2.00
DA - 27,26,25,24,23	0.013	0.00380	2.50
DA - 27,26,25,24,23,21	0.013	0.00580	2.50
DA - 27,26,25,24,23,21,20	0.013	0.00860	2.50
DA - 22	0.013	0.00690	2.00
DA - 19	0.013	0.00700	2.00
DA - 22,19,18	0.013	0.00480	2.50
27,26,25,24,23,22,21,20,19,	0.013	0.00490	3.50
DA - 16	0.013	0.00500	2.00
Basin - 1	0.013	0.00840	1.00
Basin - 2	0.013	0.00190	1.50

Bentley Systems, Inc. Haestad Methods SolBteotleGeritzerWaster V8i (SELECTseries 1) [08.11.01.03]

11/3/2017 12:59:41 PM

27 Siemons Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Page 1 of 3

#### **Pipe Sizing - TIA Report**

Discharge (ft³/s)	Flow Area (ft²)	Velocity (ft/s)
7.80	1.21	6.46
9.70	1.82	5.34
15.30	2.27	6.74
18.70	2.81	6.66
21.80	3.72	5.86
8.10	1.54	5.25
8.80	1.70	5.18
18.70	2.89	6.47
13.50	2.04	6.62
17.10	2.48	6.91
19.70	2.62	7.51
11.80	1.83	6.46
20.70	3.01	6.88
31.10	3.98	7.82
62.90	6.56	9.59
6.20	0.93	6.70
12.10	2.39	5.05
8.20	1.15	7.14
14.60	2.68	5.44
27.10	4.71	5.76
29.10	4.03	7.23
34.30	3.91	8.77
14.20	2.16	6.57
12.00	1.88	6.38
27.20	4.13	6.59
58.10	7.11	8.18
8.20	1.60	5.12
3.48	0.74	4.67
3 98	1.36	2 92

#### **Pipe Sizing - TIA Report**

Discharge Flow Area Velocity (ft³/s) (ft²) (ft/s)

Bentley Systems, Inc. Haestad Methods SolBteatlegelitewMaster V8i (SELECTseries 1) [08.11.01.03]

